REPORT

TO THE

NEW ENGLAND DIVISION CORPS OF ENGINEERS

ON THE

WEST AND SOUTHWEST BRANCHES

OF THE

HOUSATONIC RIVER

PITTSFIELD, MASSACHUSETTS

PRELIMINARY ENGINEERING ANALYSIS

OF

NON-STRUCTURAL FLOOD DAMAGE PREVENTION

Draft.

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INTRODUCTION

The following report is a preliminary analysis of the estimated costs for non-structural flood damage prevention for structures affected by two flood levels along the west and southwest branches of the Housatonic River in the City of Pittsfield, Massachusetts.

This report evaluated residential, commercial, and apartment buildings to determine what possible methods could be employed to floodproof these structures through various non-structural techniques.

Each structure was analysed to determine what non-structural flood technique could be appropriately used to protect each structure against two distinct flood levels, the 100 year flood and the Standard Project Flood (SPF). In a number of cases non-structural techniques of floodproofing were inappropriate and impractical. It was concluded that in these cases the use of non-structural techniques could not be applied without affecting the structural integrity of the building or severely limiting the practical use of the structure. Therefore, if a structure could not be floodproofed or raised above the flood level (an acceptable floodproofing technique), the structure was categorized as requiring demolition.

Sheets 1 through 4 of the Attachment shows the impact of the two flood levels investigated and what structures would require raising or demolition. All other structures evaluated within the study area could utilize non-structural flood techniques outlined in this report.

The information included in this study is not meant to be conclusive, but rather to provide a rough guide for the preliminary analysis phase from which future decisions may be made for a later, more detailed study. All work undertaken for this investigation was performed in accordance with Contract Number DACW 33-77-0066, Work Order Number 16.

SUMMARY

In order to develop estimated costs of floodproofing individual structures located along the two branches of the Housatonic River that would be subject to two distinct flood conditions, the following procedures were used: a field survey was performed to determine the type of structure and the estimated flood inundations. The structures were grouped into residential, commercial, and apartment categories. Costs of floodproofing were estimated according to the size of the structure and the extent of inundation.

Floodproofing of residential structures consisted of providing a peripheral drainage system, waterproofing and blocking up basement walls and raising foundations, and the provision of flood shields at building entries. The extent of these measures was dependent upon basement usage and the depth of inundation. Costs were estimated using unit perimeter prices proportioned to the size of the house. Commercial structures were considered using similar measures, however, the commercial usage of the structure and the estimated extent of damage was taken into account. No costs were applied for the purpose of floodproofing free-standing garages since it was assumed that water would enter the structure during a flood and exit during the recession without causing damage, nor for demolition of garages. Other categories consisting of apartment buildings and commercial structures were also studied. In some cases, due to the physical characteristics of the apartment building or commercial property, nonstructural floodproofing techniques are not

applicable. Such structures would require that earth berms be constructed with flood gates to provide access. However, if flood walls, berms, or other conventional means of flood protection were not practical, an estimated cost for demolishing the structure was developed as part of this report.

Structures were grouped along both the west and southwest branches of the Housatonic River into river reaches. The southwest branch river reaches according to location are:

Reach 1: Railroad Bridge to Barker Street

Reach 2: Barker Street to dam opposite Gale Avenue
The west branch river reaches according to location are:

Reach 1: Tel-Electric dam to West Street

Reach 2: West Street to Columbus Avenue

Reach 3: Columbus Avenue to Linden Street

Reach 4: Linden Street to Pontoosuc Avenue

In all cases which were investigated, approximately 63 percent were conventional one family residential dwellings. The majority of these structures required Type A, B, C, or D floodproofing (see PROCEDURE section) at an average cost per structure of thirteen and eighteen thousand dollars (\$13,000 & \$18,000) for the two flood conditions studied. About 21 percent of the structures studied were apartments and they required an average floodproofing cost per structure of twelve and sixteen thousand dollars (\$12,000 & \$16,000) for the two flood conditions. Commercial structures constituted about 16 percent

of the cases studied, and their floodproofing cost averaged one hundred twelve and one hundred fifty-eight thousand dollars (\$112,000 & \$158,000) for the two flood conditions.

REPORT

I. PROCEDURE

A. FIELD STUDY

The field study identified all structures which would be affected by the 100 year frequency and the Standard Project Flood (SPF).

Structures were visually field evaluated for general condition, usage, size, first floor elevation, type of foundation and basement. Elevations were obtained from the Corps of Engineers photogrammetric topography plan. This plan had contours at five foot increments creating the need for estimating elevations. Photographs were taken and all observed changes from the topographic plan were recorded. It was observed during the field study that structures have been removed and new structures added since the date of the original plan. These changes have been reflected on the topography plan enclosed.

The above data was then compiled with respect to the elevation of the estimated flood surface (of the 100 year flood and the SPF) for each structure. These elevations were obtained from flood profiles developed by the U.S. Army Corps of Engineers, New England Division.

The depth of inundation was then estimated and the proper classification of floodproofing determined for each case and for each flood condition.

B. FLOODPROOFING CLASSIFICATION

Generally, floodproofing for residential, apartment, and commercial structures was divided into seven major categories. The categories were determined by the depth of inundation and the basement usage. The

first three categories (Types A, B and C) were applied to structures where the proposed depth of inundation is below the first floor. Type D applies to cases where the flood waters are less than three feet above the first floor and all unusual cases were considered in a separate category (Type E) with each structure evaluated on an individual basis. Type F category applies to structures receiving no floodproofing. A final category (Type G) involves the case where the depth of inundation is greater than three feet above the first floor or there exists no practical means of floodproofing the structure. For this category, demolition of the structure would be required. Structures which could not be raised or floodproofed by conventional methods were listed under this category.

The following represents a breakdown of each category indicating the measures to be taken and the assumptions used in classification:

TYPE A

Type A floodproofing is used for structures that have unfinished basements with no storage. Type A floodproofing techniques consist of digging a trench in the basement floor and installing a drainage system to remove the water that accumulates. The trench would be located around the periphery of the basement approximately two feet inward from the walls. The trench should have a depth of about two feet. A system of six-inch diameter vitrified clay pipes leading to a sump hole containing a pump would be installed within the bottom of the trench and backfilled

with crushed stone. The sump pump would require a separate electric outlet and would be connected to an outside hose which would divert water away from the basement. The top four inches of the trench would be finished concrete in order to restore the basement to its original condition. Twelve (12) structures under the 100 year flood and six (6) under the SPF were placed in this category.

TYPE B

Type B floodproofing is used for structures that have finished basements with storage but no living accommodations. Houses in fair to excellent condition having basements were classified within this category. The procedures to be followed for this type of floodproofing consist of the Type A drainage system, as well as waterproofing of the outside of the basement walls. Waterproofing basement walls would require a trench be excavated around the outside periphery of the structure. The exposed basement walls would then be cleaned and waterproofing applied. The trench would be backfilled and compacted and the yard restored to its original condition. For the 100 year and the Standard Project Flood, 145 and 93 structures respectively required this method of floodproofing.

TYPE C

Type C floodproofing is applied to structures having finished basements being used for living quarters and storage. This technique requires the same measures as Type B with the additional precaution of blocking up all windows and doors. This would require the removal of

existing doors and windows, to be replaced with block masonry. This measure could cause problems with regard to local fire and building codes. Such related problems were not formally addressed within the scope of this report. Twenty-one (21) structures under the 100 year flood and fifteen (15) under the SPF were placed in this category.

TYPE D

Type D floodproofing is used for structures having basements which would receive a depth of inundation above the first floor. This technique would consist of the Type C technique with the additional measure of raising the foundation above the flood elevation. The raising of the foundation would require the structure be lifted by hydraulic jacks and temporarily supported by cribbing. All utility lines would be disconnected prior to this operation. The foundation would then be extended to the new elevation of the structure and the utilities reconnected. After the new foundation is completed, the jacks can be removed and the house and yard restored to their original condition. In order to perform this operation, it may be necessary to evacuate the occupants for approximately two to four weeks while construction is being completed. Forty (40) structures under the 100 year flood and seventy-four (74) structures under the SPF came under this category.

TYPE E

Type E floodproofing applies to residential and commercial cases which have a depth of inundation above the first floor, but cannot be

floodproofed by any of the already mentioned procedures. These structures were examined on an individual basis with explanations and costs presented in Appendix A. In all cases, a more detailed engineering investigation would be required prior to construction.

For those cases requiring flood shields, it should be noted that the shields are only installed during a flooding condition. Therefore, suitable warning time would have to be provided prior to a flood. Without this warning time, the structures would have limited protection which could result in substantial damage to the structures and their contents. For the 100 year flood and the SPF, sixteen (16) and thirty-four (34) structures respectively were grouped into this category.

TYPE F

Type F applies to structures which will receive no formal floodproofing under this study. Such structures are those which are not affected by either flooding condition or those for which the usage of the structure does not dictate formal floodproofing. One hundred three (103) structures and thirty-five (35) structures were grouped under this category for each of the two flooding conditions respectively.

TYPE G

Buildings that are placed into this category are structures that could not be floodproofed by any of the methods previously discussed.

Buildings placed into this category are structures which would receive a depth of inundation above or in excess of three feet above the

first floor, or because of the structures' construction or intended use the application of the floodproofing methods discussed would affect the building's structural integrity or severely limit the practical use of the building. Structures categorized under Type G were classified, for the purpose of this study, as requiring demolition.

C. COSTS

The costs for Types A, B, and C were obtained based on a unit cost per perimeter foot. The calculations used in formulating these costs are shown in Appendix B and the final rounded-off values are presented in Table III. Costs for Types A, B and C floodproofing were obtained by multiplying the perimeter by the unit cost. Type D floodproofing is estimated assuming Type C costs plus an additional lump sum based on the estimated cost of raising the foundation. Type E floodproofing is estimated on an individual basis with the explanation presented in Appendix A according to the footnote number. Type F floodproofing requires no formal procedure and therefore, no cost is assumed for this study. Structures listed under Category G and the associated demolition costs are presented in Appendix A and in Table I. Demolition costs are based upon \$0.10 per cubic foot, which is added to the estimated fair market value for total demolition cost (not including costs for the relocation and resultant social impact upon apartment tenants).

Since certain variables making up the floodproofing and foundation raising costs are related to the size of the building, different unit prices for different size buildings are presented in Table III. The

raw unit costs used in these calculations are based on typical values from the Robert Snow Means Company, Inc., 1979 Building Cost Data publication as well as estimates provided by local contractors and our own engineering judgment. Final costs were derived from the raw costs with operational adjustments. These adjustments consist of an additional 10 percent for contingencies or unforeseen construction difficulties, an additional 10 percent for general contractors' overhead and profit, and 10 - 20 percent for engineering and survey fees. For this study it was assumed that the engineering and survey fee would be 20 percent for Types A, B and C floodproofing, and 10 percent for foundation raising (Type D) where the experience of the contractor is most critical to the success of the operation.

II. RESULTS OF NON-STRUCTURAL METHODS STUDIED

Table I lists each structure examined during this investigation.

Contained within this Table is the address, a code system describing the structure, the effect of the proposed flooding upon the structure and the recommended floodproofing technique and its cost. Also included in the Table is the estimated first floor elevation and perimeter of each structure. Commercial buildings or industrial buildings examined may also contain a footnote number. These numbers refer to Appendix A where the structure's description, usage and recommended floodproofing technique is presented on an individual basis. Within Table I, the column headed "Type" refers to a classification code system used in

describing the structure. The first letter of the code system refers to the primary use of the structure; "C" refers to a commercial structure; and "A" refers to an apartment building or complex containing more than four individual units. "R" refers to residential structures; and "I" refers to an industrial building or complex. The adjacent second letter is used to define the primary material from which the building is constructed. "W" refers to wood, "B" to block and "M" to metal. The next number immediately following these two letters refers to the number of stories. A number containing "1/2" refers to a structure containing a finished or semi-finished attic apparently used for living or storage. The final number refers to the basement. A zero ("0") indicates no basement or slab-on-grade. A "l" refers to a crawl-type basement; a "2" refers to an unfinished basement; a "3" refers to a finished basement; a "4" refers to an unfinished basement with an enclosed garage; a "5" refers to a finished basement with an enclosed garage; and a "6" refers to an unfinished basement with storage. For all structures whose overall condition is rated poor, a "*" follows the above code.

The column headed "Depth of Inundation" refers to the depth of water, above the basement floor, during each of the two proposed flood conditions examined. The column headed "Depth of Water above F.F." refers to the total height of water above the estimated first floor grade during the two floods. A zero in this column indicates that the water will not reach the first floor. In the case of a structure with a slab-on-grade foundation, the two columns will have the same value. The column headed

"Cost in Thousands" refers to the estimated costs for floodproofing each structure or the costs of demolition if applicable. The column headed "Estimated Market Value" is our estimation of the fair market value of those structures, including the land, that fall in the demolition category.

Table II represents a breakdown of all cases considered, grouping the structures according to Reach Number. Each structure within the Reach is further analyzed according to the type of structure, the size and the recommended floodproofing technique.

Table III represents the estimated cost of different floodproofing techniques. Table values were obtained according to procedures described in Section I-C of this Report.

Tables IV and V represent the estimated cost for each Reach category respectively. Values used in these Tables were obtained from tabulation of quantities presented in Table I.

DESCRIPTION OF SYMBOLS USED IN TABLE I

	ADDRESS	 ,			Base Photogra Topograp	MMETRIC (FLOOD ABOVE I	TH OF WATERS BASEMEN FLOOR F	FL WAT T AB OR FI	TH OF OOD PERS OVE RST OOR		ĵ	ESTIN COST FLOODPE	OF ROOFING	REFERS TO APPENDIX A DESCRIPTION
	STREET	HOUSE	TY	PE	ESTIMATED FF ELEVATION	ESTIMATED PERIMETER LF	INUNE	TH OF PATION	ABO/	TH OF TER /E F.F.	FLOOD TECH	POSED PROOF NIQUE	\$ 1000		FOOTNOTE #
							1936	S.P.E	1936	S.P.F.	1936	S.P.F.	1936	S.P.F	
	Adams Street	35	CW-	2-2*	395	160	16	20	5	9	G	G	1.5	1.5	
-15	Mechanic Street	52	CB-	1-2	397	570	7	10	0	0	Е	. E	3.3	3.3	1.
'	Cotton Street	36	RW-	2-2	407	160	3	7	0	0	В	В	9.1	9.1	
	Category A- APARTMENT C- COMMERCIAL R- RESIDENTIAL I- INDUSTRIAL Predominant Structural Mater: B- BRICK OR BLA W- WOOD M- METAL			Ba	Poor Conditiusement Usage 0- NO BASEME 1- UNFINISHE 2- FINISHED 3- FINISHED 4- UNFINISHE 5- FINISHED 6- UNFINISHE per Of Floors	ENT ED BASEMENT, I BASEMENT UI BASEMENT LI ED BASEMENT VI BASEMENT WI CD BASEMENT VI	TH STOR VING AR WITH EN TH ENCL	AGE EA CLOSED OSED GA		TY TY TY TY TY TY	IPE A- IPE B- IPE C- IPE D- IPE E- IPE F- IPE G-	PERIPHI TYPE A OF BASI TYPE B WINDOWS TYPE C UNUSUAL NO FORM	PLUS WEMENT WEMENT WE PLUS FOR CONDINGROUS WALFLOR CONDINGROUS WALFLOR CONTINGROUS WATCH	DRAINAGI NATERPRO NALLS NLOCKING RAISING TIONS OODPROON	FOUNDATION - SEE FOOTNOTE FING TECHNIQUE RACTICAL

TABLE I SW Branch REACH NO. 1 DEPTH OF WATER DEPTH OF PROPOSED FLOOD PROOF TECHNIQUE COST IN ESTIMATED **ESTIMATED ESTIMATED** INUNDATION HOUSE \$1000.00'S STREET TYPE F.F. ABOVE F.F MARKET PERIMETER FOOTNOTE # NO ELEVATION VALUE LF 100 YR S.P.F. 100 YR S.P.F. 100 YR S.P.F. 100 YR S.P.F. 0.0 15.6 992 160 0 2 0 2 F D Barker Road 115 RW-1-1-16-

	SW Branch					7	ABLE	I					RE.	ACH NO.	2
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER		TH OF TER E F.F	E	TH OF ATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET	\$ 1000	•
				ELEVATION	LF	100 YR	S.P.F.	IOO YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.
	Barker Road	114	RW-2-6	995	110	0	0	0	7		F	G	\$ 38,000	0.0	39.9
	(Armory Garage) Cadwell Road (Two Buildings) Cadwell Road	106	CB-1-0 CW-2-6	987 982	250 126	0	7	0	7		F G	G G	120,000	0.0	123 . 9 85 . 0
Ī	Cadwell Road	111	RW-1 ¹ 2-2	985	120	2	9	10	17		D	G	30,000	13.8	32.3
Ì	Cadwell Road	116	RW-2-3	982	124	5	12	13	20		G	G	31,000	33.4	33.4
ᆚ	Cadwell Road	120	RW-1-4	982	160	5	12	13.	20		G	G	40,000	42.1	42.1
7-	Cadwell Road	123	RW-1-3	985 `	132	2	9	10	17		D	G	33,000	14.2	34.4
ļ	Cadwell Road	127	RW-1½-5	987	96	0	7	8	15		С	G	29,000	6.5	30.0
	Cadwell Road	130	RW-1-5	982	156	5	12	13	20		G	G	38,000	40.0	40.0
	Cadwell Road	131	RB-1-5	987	100	0 ^	7	8	15		С	G	29,000	6.7	29.8
	Cadwell Road	136	RW-1 ¹ 2-3	985	112	2	9	10	17		D	G	30,000	13.5	31.6
	Cadwell Road	141	RW-1⅓-6	987	124	0	7	8	15		В	G	31,000	7.3	32.9
	Cadwell Road	190	RW-2 ¹ 2-6	983	128	4	11	1.2	19		G	G	32,000	35.1	35.1
-17-	Cadwell Road	200	CW-1-0	981	140	6	1.3	6	13		G	G	35,000	36.2	36.2
	Cadwell Road	203	RW-2-6	984	100	3	10	11	18		G	G	29,000	30.6	30.6

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	SW Branch	·				7	ABLE	I					RE/	ACH NO.	2
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER	DEP1 WA ABOV	TH OF TER E F.F	i .	TH OF PATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET	cos1	
				ELEVATION	ĻF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.
	Cadwell Road	207	RW-2-6	982	124	5	12	13	20		G	G	\$ 31,000	33.4	33.4
	Cadwell Road	229	RW-1½-6	987	136	0	7	. 8	15		В	G	34,000	8.5	36.3
	Cadwell Road	231	RW-1 ¹ 2-3	987	108	0	7	8	15		С	G	30,000	7.1	31.5
	Cadwell Road	233	RW-2-6	983	112	4	11	12	19		G	G	30,000	32.0	32.0
	(Access behind 13 Cadwell Road	6 Ca	iwe11) RW-2½-6	978	104	9	16	1.7	24		G	G	30,000	32.0	32.0
-18-									,					·	
န	Gale Avenue	2	RW-l¹½-6	978	130	9	16	17	24		G	G	32,000	34.1	34.1
	Gale Avenue	8	RW-2 ¹ 2-2	983	140	4	11	12	19		G	G	35,000	38.7	38.7
	Gale Avenue	16	RW-1 ¹ 23	983	160	4	11	12	19		G	G	40,000	43.2	43.2
	Gale Avenue	22	RW-2-2	984	140	3	10	11	18		G	G	35,000	38.1	38.1
	Gale Avenue	27	RW-2-2	988	152	0	6	7	14		В	G	38,000	8.8	41.6
	Gale Avenue	28	RB-2-6*	993	120	0	1	2	9		A	D		4.2	13.8
	Gale Avenue	32	RB-1½-3	998	160	0	0	0	4		С	С		9.8	9.8
	Gale Avenue	33	RW-2-6	995	124	0	0	0	7		F	В		0.0	7.3
		!								,	·	<u>.</u>		 	
	Greendale Avenue	4	RW-1-0	983	70	4	11	4	11		G	G	25,000	25.5	25.5

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TABLE I SW Branch REACH NO. 2 DEPTH OF PROPOSED DEPTH OF COST IN FLOOD PROOF **ESTIMATED ESTIMATED** WATER **ESTIMATED** INUNDATION HOUSE \$1000.00'S TECHNIQUE STREET TYPE F.F. ABOVE F.F. FOOTNOTE ## MARKET PERIMETER NO **ELEVATION** VALUE LF 100 YR 1 S.P.F. 100 YR S.P.F. 100 YR | S.P.F. IQQ YR S.P.F. 0 0 5 С C 5.7 5.7 21 RW-1-3 997 80 0 Greendale Avenue 5.1 5.1 995 0 0 0 7 В В 22 RW-23-6 80 Greendale Avenue 7.1 0.0 24 RW-2-4 1000 120 0 0 0 2 F В Greendale Avenue 5 0.0 7.1 0 Ð F Greendale Avenue 31 RW-2-6 997 120 0 В 0 0 2 F 0.0 7.1 1000 120 Ω В Greendale Avenue 34 RW-1-4 ö 1000 0 n 2 F В 0.0 8.3 48 AW-2-4 144 Greendale Avenue (WBEC Radio) E 21.0 32.0 211 CB-1-0 985 350 2 9 2 9 1 E Jackson Street 0 1 8 C C 8.3 8.3 994 132 0 Jackson Street 217 RW-13-3 \$ 33,000 7.8 35.7 5 12 G Jackson Street 223 RW-2-2 990 132 0 4 В 128 0 3 11 С D 8.1 14.2 991 4 W. Housatonic St. 418 RW-13-3 0.0 7.1 0 0 4 F В 998 120 0 W.Housatonic St. 421 RW-2-2 6.2 13.0 0 1 2 9 В Ð 422 RW-2-6 993 100 W. Housatonic St. 38,000 8.5 41.4 7 G 0 15 В 427 RW-2-2 987 148 W. Housatonic St. G 40,000 44.4 44.4 5 12 G .982 216 5 12 W. Housatonic St. 433 RB-1-0

	SW Branch					7	ABLE	I					RE	ACH NO.	2
	STREET	HOUSE NO	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER		TH OF TER E F.F	DEPT	H OF ATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	\$ 1000).00's
Į	· .			ELEVATION	ĻF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	\$.P.F.
	"Big N"Shopping P W.Housatonic St.		CB-1-0	979	2020	8	15	8	15	2	E	E	\$2,550,000	3060.	3060.
	McDonalds W.Housatonic St.		CB-1-0	976	240	11	18	11	18	. 3	Е	E	180,000	187.2	187.2
	W.Housatonic St.		RW-1-0	976	116	11	18	11	18	4	E	E	29,000	30.3	30.3
Ì	Fitch Motel - 22 W.Housatonic St.	1	CW-1-0	976	520	11	18	11	18	4	E	E	330,000	343.2	343.2
	Diner W. Housatonic St.		CW-1-0	977	112	10	17	10	17	4	Е	E	10,000	11.2	11.2
-20	Shell Station W.Housatonic St.	484	CB-1-0	975	188	12	19	12	19	4	Е	E	33,000	51.3	51.3
20-	Sunoco Station W.Housatonic St.		CB-1-0	975	340	12	19	12	19	5	Е	E	90,000	114.0	114.0
	W.Housatonic St.	490	RB-2-3	978	102	9	16	17	24		G	G	29,000	30.6	30.6
	W.Housatonic St.	541	RW-2-2	993	104	0	1	2	9		В	D		6.2	13.2
	W.Housatonic St.	565	RB-1-3	988	120	0	6	7	14		С	G	30,000	7.6	31.4
	W.Housatonic St.	572	RW-2-2	986	132	1	8	9	16		D	G	33,000	14.3	35.7
ļ	W.Housatonic St.	573	RW-2-5	984	100	3	10	11	18		G	G	29,000	30.6	30.6
Ĭ	W.Housatonic St.	577	RW-1½-2	983	120	4	11.	12	19		G	G	30,000	31.8	31.8
	W.Housatonic St.	578	RW-2-6	982	100	5	12	13	20		G	G .	29,000	30.6	30,6
	Mobil Station W.Housatonic St.	581	CB-1-0	982_	160	5	12	5	12	6	E	Е	24,000	41.4	41.4
	W.Housatonic St.	582	RW-1-3	981	148	6	13	14	21		G	G	38,000	40.1	40.1

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STREET	HOUSE NO	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	W	TH OF ATER VE F.F	1	TH OF DATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	cos \$100	
	,,,,		ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.
W.Housatonic St.	586	RW-1 ¹ 2-2	980	132	7	14	15	22		G	G	\$ 33,000	35.2	35
W. Housatonic St.	590	RB-1-0	979	118	8	15	8	15		G	G	29,000	30.3	30
W.Housatonic St.	591	RW-2 ¹ 2-2	985	100	2	9	10	17		D	G	29,000	13.0	30
W.Housatonic St.	596	RW-1 ¹ 2-2	983	120	4	11	12	19		G	G	30,000	31.8	31.
W.Housatonic St.	595 -597	AB-2-6	985	160	2	9	10	17		D	G	40,000	15,6	44
Body Shop W.Housatonic St.	607	CB -1 -0	982	200	5	12	5	12	7	Е	Е	38,000	43.0	43
Tire Shop W.Housatonic St.	615	CB-1-0	983	240	4	11	4	11	8.	Е	E	54,000	61.2	61
Woodleigh Road	20	RB-1 ¹ ₂ -3	986	120	1	8	9	16		D	G	30,000	13,8	31
Woodleigh Road	30	RB-1-6	990	160	0	4	5	12		В	G	40,000	9.1	42
Woodleigh Road	40	RW-1 ¹ 2-2	1000	120	0	0	0	2		F	В		0.0	7
Zoar Street	9-11	AB-2 ¹ 2-6	982	120	5	12	13	20		G	G	30,000	32.7	32
Zoar Street	15 ~ 17	AW-2 ³ 2-2	989	120	0	5_	6	13		В	G	30,000	7.1	32

TABLE I W. Branch REACH NO. 1 DEPTH OF PROPOSED FLOOD PROOF DEPTH OF COST IN ESTIMATED **ESTIMATED** WATER **ESTIMATED** INUNDATION \$1000.00's HOUSE STREET TYPE F.F. ABOVE F.F TECHNIQUE FOOTNOTE # MARKET PERIMETER NO ELEVATION LF VALUE 100 YR S.P.F. 100 YR | S.P.F. IOO YR S.P.F. 100 YR S.P.F. Apt. Complex, 24 Apts./Unit * 0.0 0.0 Deering Street 1006 N/A 0 0 0 0 F \boldsymbol{F} 10 AW-3-0 0.0 0.0 0 0 0 0 F F 20 AW-3~0 1003 N/A Deering Street 30-Deering Street 0.0 AW-3-0 1002 N/A 0 0 0 0 F F 0.0 * 1000 0 0 0 0 F 0.0 0.0 Deering Street AW-3-0 N/A F 50-* Deering Street AW-3-0 997 N/A 0 0 0 0 F F 0.0 0.0 52 60-Deering Street 0 0.0 0.0 62 AW-3~0 999 N/A 0 0 0 F F Northeast Utilities Service Center 0.0 F 0.0 --- CB-1-0 0 0 0 West Street 1000 N/A

TABLE I W. Branch REACH NO. 2 PROPOSED FLOOD PROOF DEPTH OF DEPTH OF COST IN **ESTIMATED ESTIMATED** WATER **ESTIMATED** INUNDATION HOUSE \$1000.00'S TECHNIQUE F.F. ABOVE F.F. STREET TYPE FOOTNOTE ## MARKET PERIMETER NO ELEVATION VALUE LF 100 YR S.P.F. 100 YR | S.P.F. 100 YR S.P.F. 100 YR | S.P.F. 9 ′ Columbus Avenue 297 AB-3-6 1002 240 0 1 3 В 13.5 18.0 D Riverside Cafe 1001 120 0 2 10 . 9 В Е \$ 14,000 7.1 16.3 Columbus Avenue 301 CB-2-6 4 323 RW-2-3 1000 140 0 3 5 11 C 8.7 14.7 Columbus Avenue D 8.1 8.1 Columbus Avenue 329 RW-2-2 1004 140 0 0 1 7 В В 3 F 0.0 9.8 Columbus Avenue 337 RW-2-3 1008 160 0 0 0 C 9.8 0 0.0 Dewey Avenue 73 RW-2-3 1008 160 0 0 3 F С 110 4 18 6,000 7.9 7.9 Dewey Avenue 73R RW-2-6* 993 10 12 G G 79 1004 110 0 0 1 .77 В 6.6 6.6 Dewey Avenue RW-2-2 В 170 5 C 0.0 10.3 West Street 306 RW-2-3 1006 0 0 0 310 RW-2-3 1004 130 0 0 7 C С 8.2 8.2 1 West Street

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	W.Branch		÷			. 7	ABLE	I					RE	ACH NO.	3
	STREET	HOUSE	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER		H OF IER E F.F		TH OF DATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	\$ 1000	
				ELEVATION	LF	100 YR	S.P.F.	100 YF	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	\$.P.F.
	Bradford Street	275- 277	AW-2-6	1008	200	0	0	0	3		F	В		0.0	11.3
	Bradford Street	279	RW-2-0	1006	N/A	0	0	0	0	. *	F	F		0.0	0.0
	Bradford Street	280	RW-2-2	1008	160	0	.0	0	3		F	В	·	0.0	9.2
	Bradford Street	283	RW-2-2	1008	150	0	0	0	3		F	В		0.0	8.6
	Bradford Street	282- 284	AW-2½-6	1008	210	0	0	0	3		F	В		0.0	11.8
-24-	Bradford Street	287 288~	RW-1½-0	1006	N/A	0	0	0	0	*	F	F		0.0	0.0
1	Bradford Street	t 1	AW-2½-1	1007	N/A	0	0	0	0	*	F	F		0.0	0.0
						<u> </u>					<u></u>			<u> </u>	
	West Side Fish Ma Columbus Avenue	rket 300	CB-1-0	1000	130	0	3	0	3	10	F	E	\$ 16,000	0.0	18.6
	Columbus Avenue	314- 316	AW-2-2	1000	190	0	3	7	11		В	D		10.7	16.7
	Columbus Avenue	322	RW-2-6	1000	140	0	, 3	7	11		В	D		8.1	14.7
	Columbus Avenue	326 - 328	AW-2-3	1004	210	0	0	3	7		С	С		12.8	12.8
	Columbus Avenue	334	CB-1-0	1008	N/A	0	0	0	0	<u>*</u>	F	_F		0.0	0.0
	Dewey Avenue	96	RW-2 ¹ 2-6	1005	200	0	0	2	6		В	В		11.3	11.3
	Milton's Place Dewey Avenue	97	CB-1-0	998	90	1	5	1.	5	11	E	G	8,000	8.5	8.5

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	W. Branch					. 7	ABLE	I					RE	ACH NO. 3	
	STREET	HOUSE	ЗЧҮТ	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER	DEPT WAT ABOV	ER		TH OF PATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	COST IN \$1000.00	's
				ELEVATION	LF.	IOO YR	S.P.F.	100 YR	S.P.F.	<u> </u>	100 YR	S.P.F.	VALUE	100 YR S.P	, F,
	Dewey Avenue	102	RW-2-1	996	250	3	7	6	10	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	D	G	\$ 24,000	18.0 33	3,8
	Dewey Avenue	107	RW-2-6	996	120	3	7	11	15		Q	G	15,000	13.8 17	7.3
	Dewey Avenue	110	RW-2-1	1000	160	0	3	2	6		A	D		5.5 15	5.6
1	Dewey Avenue	111	RW-2-6	996	130	3	7	11	15		D	G	15,000	14.3 17	7.6
	Dewey Avenue	113	RW-2-1	997	100	2	6	5	9		a	G	15,000	13.0 16	6.6
,	Dewey Avenue	114	RW-2-6	998	150	1	5	9	13		D	G	18,000	15.1 21	1.7
25-	Dewey Avenue	113	RW-2-2	999	230	0	4	8	_12		В	G	43,000	13.0 51	1.2
	Dewey Avenue	122	CB-2-6	1004	170	0	0	3	7		В	В		9.6	9.6
	Dewey Avenue	125	RW-2-1*	996	100	3	7	6_	10		D	G	8,000	13.0	9.6
	Dewey Avenue	129	RW-2-3	998	120	1	5	9	13	,	D	G	15,000	13.8 17	7.3
	Dewey Avenue	135- 137	AW-2-6	998	160	1	5	9	13		D	G	21,000	15.6 25	5.0
	Dewey Avenue		RW-2 ¹ ₂ -2	998	140	11	5	9	13		D	G	16,000	14.7 19	9.7
	Dewey Avenue	143- 145	AW-2 ¹ 2-2	998	140	1	5	9	13		D	G	16,000	14.7 19	9.7
	Dewey Avenue	147	RW-2 ¹ 2-2	998	150	1	5	9	13		D	G	18,000	15.1 2	1.7
	Dewey Avenue	154	CB-2-0	1000	160	0	3	0	3	1.2	F	E		0.0	5.0
	Dewey Avenue	155	RW-2-2	998	130	1	5	9	13		D	G	15,000	14.3 1	7.6

	W. Bra	anch					T	ABLE	I					RE	ACH NO.	3
	S1	TREET	HOUSE	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	DEP1 WA ABOV	H OF IER E F.F	Į.	H OF ATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET		NI T 0.00'S
					ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	IOO YR	S.P.F.
	Dewey	Avenue	159	RW-2-1	998	130	1	5	4	8		D	G	\$ 20,000	14.3	22.6
	Dewey	Avenue	165	CB-1-0	995	280	4	8	4	8	·	G	G	38,000	42.9	42.9
	Dewey	Avenue	166	RW-1½-2	1001	130	0	2	6	10		В	Đ		7.6	14.3
	Dewey	Avenue	168	AW-2-6*	1004	200	0	0	3	7		В	В		11.3	11.3
	Dewey	Avenue	169	RW-2-1	998	120	1	5	4	8		D	G	13,500	13.8	15.8
	Dewey	Avenue	172- 174	AW-2-6	1003	240	0	0	4	8		В	В		13.5	13.5
-26-	Dewey	Avenue	173	RW-2½-6	1000	180	0	3	7	11		В	D		10.2	16.4
	Dewey	Avenue	175	RB-2-0	1001	120	0	2	0	2	13	F	E	14,000	0.0	16.3
ĺ	Dewey	Avenue	181	RW-2 ¹ ₂ -1	1001	190	0	_2	1	5		A	D		6.5	16.7
	Dewey	Avenue	185- 187	AW-2-1	1001	160	0_	2	1	5		A	D		5.5	15.6
	Dewey	Avenue	189	RW-2-6	1001	140	0	2	6	10		В	D		8.1	14.7
	Dewey	Avenue	192	RW-2-1	1005	120	0	0_	0	1		F	A		0.0	4.2
	Dewey	Avenue	193	RW-2-1	1001	170	0	2	1	5		A	ā		5.8	16.0
	Dewey	Avenue	194	RW-2-2	1005	140	0	0	2	6		В	В		8.1	8.1
į	Dewey	Avenue	198	RW-2-2	1005	130	0	0	2	6		В	В		7.6	7.6
	Dewey	Avenue	199	RW-2-1	1001	150	0	2	1	5		A	D		5.2	15.1

	W. Branch					7	ABLE	I					RE	ACH NO.	3
	STREET	HOUSE	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER		H OF TER E F.F		TH OF ATION	FOOTNOTE #	l FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	COST \$ 1000	T IN 0.00's
				ELEVATION	ŁF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.
	Dewey Avenue	203	RW-2-6	1001	180	0	2	6	10		В	D		10.2	16.4
	Dewey Avenue	204	AW-3-6	1007	210	0	0	0	4		F	В .		0.0	11.8
	Dewey Avenue	205- 207	AW-2-1	1001	140	0	2	1	5		A	D		4.9	14.7
							!								
	John Street	10	RW-2-0	1006	N/A	0	0	0_	0	*	F	F		0.0	0.0
	John Street	14	RW-2-6	1006	160	0	0	1	5		В	В		9.2	9.2
-27-	John Street	20 24-	RW-2-2	1005	200	0	0	2	6		В	В		11.3	11.3
	John Street	24-	AW-2-6	1004	180	0	0	3	7		В	В		10.2	10.2
	John Street	43- 45	AW-2-6	1011	N/A	0	0	0	0_	*	F	F		0,0	0.0
	John Street	44	RW-2-2	1003	200	0	0	4	8		В	В		11.3	11.3
	John Street	46	RW-2-6	1000	130	0	3	7	11		В	D		7.6	14.3
	John Street	47	RW-2-6	1010	170	0	0	0	1		F	В		0.0	9.6
	John Street	50- 52	AW-2-2	1000	180	0	3	7	11		В	D		10.2	16.4
	John Street	51 - 53	AW-2-1	1008	N/A	0	0	. 0	_0	*	F	E	· · · · · · · · · · · · · · · · · · ·	0.0	0.0
	John Street	54	RW~2 ¹ 5~6	1000	100	0	3	7	11		В	D		6.2	13.0
	Berkshire Bakery John Street	56	CW-2-0	995	424	4	8	4	8		G	G.:	\$112,000	140.1	140.1

	W. Branch		•			. T	ABLE	I						RE	ACH NO.	3
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER L F	DEPT WAT ABOVE	H OF FER F.F	1	TH OF DATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE		STIMATED MARKET VALUE	cos-	T IN 0.00'S
- [L.r	IOO YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.			100 YR	S.P.F.
	John Street	62 - 68	AW-2-6	1000	200	0	3	7	11		В	D			11.3	17.1
	John Street	62- 68R	CW-2-1	998	240	11	5	4	8		D	G	\$	2,000	18.0	11.0
	John Street	70	RW-2 ¹ ₂-6	1000	150	0	3	7	11		B	D			8.6	15.1
	John Street	71	RW-2-2	1002	150	0	1	5	9		В	D			8.6	15.1
	John Street	72	RW-2-6	1000	110	0_	3	7	11		В	D			6.6	13.4
-2	John Street	75	RW-2-6	1003	140	0	0	4	88		В	В			8.1	8.1
28-	John Street	76	RW-2-6	1000	100	Ó	3	7	11		В	D			6.2	13.0
	John Street	77 - 79	AW-2-6	1004	200	0	0	3	7		В	В			11.3	11.3
	John Street	78 - 80	RW-2 ¹ 2-2	1000	140	0	3	7	11.		В	D			8.1	14.7
	John Street	82	RW-2 ¹ 2-6	1000	150	0	3	7	11		В	Ď		<u> </u>	8.6	15.1
	John Street	86	RW-2-1	1000	160_	0	3	2	6		A	D			5.5	15.6
	John Street	87 - 89	AW-2-1	1004	150	0	0	0	2		F	A		· 	0.0	5.2
	John Street	90	RW-2-0*	999	160	0	4	0	4		F	G	2	22,000	0.0	26.0
	John Street	94	RW-2-1	1000	100	0	3	2	6		A	D			3.6	13.0
	John Street	95	RW-2-2	1007	140	0	0	0	4		F	В	<u> </u>		0.0	8.1
	John Street	96	RW-2-6	1000	245	0	3	7	11		В	D	<u> </u>		13.8	18.0

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	W. Branch					7	ABLE	I					RE	ACH NO.	3	
	STREET	HOUSE NO	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	DEP1 WA ABOV	TH OF TER E F.F		TH OF DATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	\$ 1000		
		110		ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	IOO YR	S.P.F.	
	John Street	99- 101	AW-2-6	1004	225	0_	0	3	7		В	В		12.7	12.7	
	John Street	100	RW-2-6	1000	155	0	3	7	11		В	D		8.9	15.3	
	John Street	104 107-	RW-2-2	998	150	1	5	9	13		D	G	\$ 22,000	15.1	25,5	
	John Street		AW-2-6	1001	150	0	2	6	10		В	D		8.6	15.1	
-29-	Nagelschmidt's Ma Linden Street		CW-2~6	1008	225	0	0	0	3		F	В		0.0	12.7	
Ť	Linden Street	177	RW-2-1	1002	155	0	1_	0_	4		F	D		0.0	15.3	
	Linden Street	181	CB-1-6	1000	100	0	3	7	11		В	D		6.2	13.0	
	Amoco Station Linden Street	198	CB-1-0	997	175	2	6	2	6	14	E	Е	48,000	68.0	68.0	
	Linden Street		AW-2-6	1004	140	0	0	3	7	Ala _n	В	В		8.1	8.1	
Ì	Linden Street	211- 213	AW-2-6	1010	230	0	0	0	. 1		F	В	<u> </u>	0.0	13.0	
							··								·	
	Prospect Street	38 39-	RW-2-6	1005	150	_0	o_	_2_	6		В	B		8.6	8,6	
	Prospect Street		AW-2½-6	1004	190	0	0	3	7		В	В		10.7	10.7	
	Prospect Street	42	RW-2-2	1003	170	0	0	4	8		В	В		9.6	9.6	
	Prospect Street	4 5	RW-2½-6*	1002	180	0_	_ 1	5_	9		В	D		10.2	16.4	

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TABLE I W. Branch REACH NO. 3 DEPTH OF DEFTH OF PROPOSED FLOOD PROOF COST IN ESTIMATED ESTIMATED WATER **ESTIMATED** INUNDATION HOUSE \$1000.00'S F.F. ABOVE F.F TECHNIQUE STREET TYPE FOOTNOTE # MARKET PERIMETER NO **ELEVATION** VALUE LF 100 YR S.P.F. 100 YR S.P.F. 100 YR S.P.F. 100 YR | S.P.F. 9.2 15.6 Prospect Street 46 RW-212-2 1000 160 0 3 7 .11 В D 3 9.2 15.6 Prospect Street 47 RW-23-6 1000 160 7 11 В D 50 RW-2½-6* 998 180 5 9 13 D G \$ 27,000 16.4 33.1 Prospect Street 1 13-Southern Avenue 15 AW-2-2 1005 180 0 0 2 6 В В 10.2 10.2 3 7 7.6 7.6 Southern Avenue 16 RW-2-2 1004 130 0 0 В В 19-2 8.1 14.7 Southern Avenue 21 AW-2-6 1001 140 0 6 10 В D 0 3 2 6 D 4.9 14.7 1000 140 Southern Avenue 20 RW-2-1 Α 15.6 28.0 5 8 24,000 Southern Avenue RW-2-1 998 160 1 D G 24 1 5 9 G 21,000 14.7 24.1 998 140 13 D Southern Avenue 28 AW-2-6

	W. Branch	TABLE I												REACH NO. 4		
	STREET	HOUSE TYPE	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER L.F	DEPTH OF WATER ABOVE F.F		DEPTH OF INUNDATION		FOOTNOTE #	PROPOSED FLOOD PROOF TECHNIQUE		ESTIMATED MARKET	COST \$ 1000	. 1	
	,	""				100 YR	5. P. F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.	
	Danforth Avenue	20	RW-2-6	1'003	180	0	1	44	9		В	D		10.2	16.4	
	Danforth Avenue	22	RW-2-3	1003	90	0	1	4	9		С	D		6.2	12.6	
	Danforth Avenue	30	RW-2-2	1003	160	0	1	4	9		В	D		9.2	15.6	
	Danforth Avenue	36	RW-2-3	1005	160	0	0	2	7		С	С		9.8	9.8	
	Danforth Avenue	42	RW-2-2	1004	130	0	0	3	8		В	В		7.6	7.6	
į,	Danforth Avenue	44- 46	AW-2-2	1002	160	0	2	5	10		В	D	-	9.2	15.6	
31-	Danforth Avenue	45	RW-2-6	1003	160	0	1	4	9		В	D		9.2	15.6	
	Danforth Avenue	48	RW-2-2	1002	130	0	2	5	10		В	D		7.6	14.3	
· ·	Danforth Avenue	50	RB-1 ¹ 2-0	1001	140	0	3	0	3	1.5	F	E		0.0	1.1	
	Danforth Avenue	65 72-	RW-2-2	1004	130	. 0	0	3	8		_В	В		7.6	7.6	
	Danforth Avenue	74	AW-2-1	996	160	3	8	6	11		D	G	32,000	15.6	36.0	
	Danforth Avenue	82	RW-2-0	996	120	3_	8_	3_	8	16	E	G	18,000	20.3	20.3	
1	Danforth Avenue	84	RW-2-1	997	120	2	7	5	10		D	G	18,000	13.8	20.3	
	Danforth Avenue	88	RW-l ¹ 2-1	998	80	1	6	4	9		D	G	8,000	12.2	8.8	
	Danforth Avenue	90- 92	AW-2-6	997	130	2	7_	10	15		D	G	21,000	14.3	23.6	
	Danforth Avenue	94- 96	AW-2-6	1000	170	0	4	7	12		В	G	36,000	9.6	40.5	

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W. Br	ranch				TABLE I								REACH NO. 4			
STREET		HOUSE NO	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	DEPTH OF WATER ABOVE F.F		DEPTH OF INUNDATION		FOOTNOTE #	PROPOSED FLOOD PROOF TECHNIQUE		ESTIMATED MARKET	COST IN \$1000.00'S		
	,	.,,		ELEVATION	LF	100 YR	S.P.F.	IOO YR	\$.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.	
Danfo	orth Avenue	100	RW-2-2	1003	180	0	1	4	9		В	D		10.2	16.4	
Danfo	orth Avenue	114	RW-2-3	1005	150	0	0	2_	7		C	C		9.3	9.3	
Danfo	orth Avenue	116	RW-2-1	1007	N/A	0	0	0	0	*	F	F		0.0	0.0	
Danfo	orth Avenue	120	RW-2-1*	1008	N/A	0	0	0_	0	*	F	F		0.0	0.0	
Danfo	orth Avenue	120F	RW-2-6	1006	130	0	0	1	66		В	В		7,6	7.6	
Danfo	orth Avenue	1.24	RW-2-6	1006	160	0	0	1	6		В	В		9.2	9.2	
Danfo	orth Avenue	125	RW-2 ¹ 2-6	1006	120	0	0	1	6	1	В	В		7.1	7.1	
Danfo	orth Avenue	128	RW-2-6	1003	150	0	1	4	9		В	D		8.6	15.1	
Danfo	orth Avenue	129	AW-2 ¹ 2-1	1000	210	0	4	2	7		A	G	\$ 41,000	7.2	47.9	
Danfo	orth Avenue	132	RW-2-6*	1000	140	0	4	7	12		В	G	25,000	8,1	28.1	
Danie	els Avenue	167	RW-2-1	1004	130	0	0	0	3		F	Α		0.0	4.6	
Danie	els Avenue	168	RW-2-2	1008	190	0	0	0	4		F	В		0.0	10.7	
Danie	els Avenue	173	RW-2-1	1003	140	0	1	0	4		F	D		0.0	14.7	
							· · · · · · · · · · · · · · · · · · ·									
Danie	el's Ave.Ext.	3	RW-2-6	999	100	0	5	8	13		В	G	13,000	6.2	14.6	

	W. Branch	Branch TABLE I											RE	REACH NO. 4		
	STREET		TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	DEPTH OF WATER ABOVE F.F		DEPTH OF INUNDATION		FOOTNOTE #	PROPOSED FLOOD PROOF TECHNIQUE		ESTIMATED MARKET	COST IN \$ 1000.00'S		
			NO	ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	IOO YR	S.P.F.	
	Daniels Ave. Ext.	5	RW-2-2	999	110	0	5	8	13	<i></i>	В	G	\$ 15,000	6.6	16.9	
	Dewey Avenue	233	RW-2-6	1005	130	0	0	2	7		В	В		7.6	7.6	
	Dewey Avenue	237	RW-2-6	1004	130	0_	0_	3	8		В	В		7.6	7.6	
	Dewey Avenue	245	RW-2-6	1004	130	0	0	3	8		В	В		7.6	7,6	
I ω ω	Dewey Avenue	247- 249	AW-2-1	1005	140	0	0	0	2		F	A		0.0	4.9	
ĩ	Dewey Avenue	253	RW-2-1	1005	160	0	0	0	2		F	A		0.0	5,5	
	Dewey Avenue	257	RW-2-6	1006	150	0_	0	11	66		В	В		8.6	8,6	
	Dewey Avenue	261	RW-2-2	1008	150	0	0	0	4		F	В		0.0	8.6	
	Dewey Avenue	265	RW-2-1	1004	140	0	0	0	3		F	A		0.0	4.9	
	Dewey Avenue	271	AW-2-1_	1001	150	0	3		-6-		F_	D		10.0	15,1	
	Dewey Avenue	272	CB-1-6	1006	210	0	0	1	6		В	В		11.8	11.8	
	Dewey Avenue	275	RW-2-2	1000	100	0	4	7	12		В	G	13,000	6.2	14,6	
	Dewey Avenue	284	RW-2-6	1005	130	0_	0_	2	7		В	В		7.6	7.6	
	Dewey Avenue	285	RW-2-6	1000	120	0	4	7	12		В	G	18,000	7.1	20.3	
	Dewey Avenue	287	RW-2-6	1000	170	0	4	7	12		В	G	36,000	9.6	40.5	

TABLE I REACH NO. 4 W. Branch DEPTH OF DEPTH OF PROPOSED FLOOD PROOF COST IN ESTIMATED : **ESTIMATED** WATER ABOVE F.F ESTIMATED INUNDATION HOUSE \$1000.00'S F.F. TECHNIQUE STREET TYPE PERIMETER FOOTNOTE # MARKET NO **ELEVATION** VALUE LF 100 YR S.P.F. 100 YR S.P.F. 100 YR S.P.F. 100 YR | S.P.F. 0 5 3 \$ 18,000 4.2 21.3 Dewey Avenue 291 RW - 2 - 1999 120 8 À G 0 7 В В 8.6 8.6 7 1005 150 0 2 Elm Vale Place RW-2-6 11 1005 110 0 0 2 7 C C 7.2 7.2 Elm Vale Place RW-2-3 7 7.2 7.2 C Elm Vale Place 15 RW-2-3 1005 110 0 0 2 С 7 7.6 7.6 0 0 2 В В Elm Vale Place 19 RW-2-6 1005 130 9.6 9.6 24 1006 170 0 0 1 6 В В Elm Vale Place RW-2-6 Fahey Beverage Company 0.0 110.0 2 2 Elm Vale Place CM-2-0 1002 580 0 0 17 F Е J. B. Paper Company 2 2 18 E 0.0 16.0 975 0 0 F Elm Vale Place CB-2-0 1002 9.2 9.2 1004 160 0 0 3 8 В В Francis Avenue 282 RW-2-6 9,9 9.9 0 Francis Avenue 283 RW-2-2 1004 175 0 3 8 В В 7.1 13.8 120 0 2 10 В Ð Francis Avenue 286 RW-2-2 1002 297-9.6 16.0 299 1002 170 0 2 5 10 В D Francis Avenue AW-2-6 301-10.2 16.4 303 1002 180 0 2 5 10 В D Francis Avenue AW-2-6

	W. Branch					ľ	ABLE	I					RE	ACH NO.	4
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER	DEPT WA ABOV	TH OF TER E F.F	I .	H OF ATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET	\$ 1000	1
				FLEAMLION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	IOO YR	S.P.F.
	King Street	8	RW-2-2	1003	170	0	1	4	9		В	D		9.6	16.0
	King Street	10- 12	AW-2-6	1001	225	0	3	6	11		В	D		12.7	18.0
	King Street	16	RW-2-2	1003	160	0	1	4	9		В	D		9.2	15.6
	King Street	20	RW-1½-6	1003	140	0	1	4	9		В	D		8.1	14.7
	King Street	24	RW-2-3	1004	150	0	0	3	8		С	С		9.3	9.3
-35-	King Street	32	RW-2-3	1006	150	0	0	1	6		С	С		9.3	9.3
Ϋ']	King Street	40	RB-2 ¹ 2-2	1006	140	0	0	1	6		В	В		8.1	8.1
	King Street	44 - 46	AW-2-2	1005_	160	0	0	2	7		В	В		9.2	9.2
	King Street	48 - 50	AW-2-6	1004	160	0_	. 0	3	8		В	В		9.2	9.2
	King Street	53	RB-2-2	1008	160	0	0	0	4		F	В		0.0	9.2
	King Street	54 - 56	AW-2-2	1003	180	0	1	4	9		В	D		10.2	16.4
	King Street	60	RW-2-3	1002	130	0	2	5	10		С	D		8.2	14.3
	King Street	64	RW-2-3	1002	110	0_	2	5	10		С	۵		7.2	13.4
	King Street	65	RW-2½-6	1008	150	0	0	0	4		F	В		0.0	8.6
	King Street	77	RW-2-2	1008	120	0_	0	0	4		F	B		0.0	7.1
	King Street	81	RW-2 ¹ 5-2	1007	160	0	0	0	5		F	В		0.0	9.2

	W. Branch					T	ABLE	I					RE/	ACH NO.	4
	STREET	HOUSE NO	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER	DEPTI WAT ABOVE	ER	DEPT		FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	\$ 1000	
ł		"		ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.
	King Street	87	RW-2 ¹ 2-2	1006	160	0	0	1	6		В	В		9.2	9.2
	King Street	91	RW-2-0	1006	N/A	0	0	0	0	*	_F_	F		0.0	0.0
	King Street	101	RW-1-5	1006	120	0	0	1.	6		C	С		7.6	7.6
	King Street	107	RW-2-0	1005	N/A	0	0_	0	0	*	F	F		0.0	0.0
	King Street	115	RW-1-0	1005	N/A	0	0	0	0	*	F	F		0.0	0.0
-36-	King Street	15.3	RW-2-0	1004	N/A	0	0	0	0	*	F	F		0.0	0.0
۲	King Street	165	RW-2½-6	1004	130	0	0	3	8		В	В		7.6	7.6
	King Street	169	RW-2-1	1007	N/A	0	0	0	0	*	F	F		0.0	0.0
	King Street	173	RW-2 ¹ ₂ -6	1008	120	0	0	0	4		F	B		0.0	7,1
	King Street	177	RW-2-1	1007	N/A	0	0	0	0	*	F	F		0.0	0.0
	King Street	181	RW-2 ¹ 2-1	1007	N/A	0	0	0	0	*	F	F		0.0	0.0
	King Street	189	RW-2-6	1009	120	0	0	0	3		F	В		0.0	7,1
	King Street	193	RW-1-2	1012	N/A	0	0	0	0	*	F	F		0.0	0.0
							·	<u> </u>							
	Leidhold Place	7~ 9	AW-2 ¹ 3-6	1003	130	_0	1	4	9		В	D		7.6	14.3
	Leidhold Place	8	RW-2-1	1003	130	0	1	0	4		F	D		0.0	14.3

	W. Branch					τ	ABLE	I		•			RE	ACH NO. 4
	STREET	HOUSE	TYPE	ESTIMATED F.F.	ESTIMATED PERIMETER		H OF ER EF.F		H OF ATION	FOOTNOTE #	PRO FLOOD TEC	POSED PROOF HNIQUE	ESTIMATED MARKET	COST IN \$ 1000.00'S
				ELEVATION	LF	IOO YR	S.P.F.	IOO YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR \$.P.F.
	Leidhold Place	10	RW-2-6	1003	160	0	1	4	9		В	D		9,2 15,6
	Leidhold Place	11	RW-2-0	1000	120	0	4	0	4		F	G	\$ 18,000	0.0 20.3
 	Leidhold Place	14	RW-2-0	1000	235	0	4	0	4		F	G	52,000	0.0 60.6
	Lenox Avenue	10	RW-2-2	1005	220	0	0	2	7		В	В		12.4 12.4
-37-	Bob's Automotive Linden Street		CB-1-0	997	130	2	7	2	7	19	Е	G	16,000	17.6 17.6
[Linden Street	202- 204 206-	AW-2-1	1000	210	0	4_	2	7		A	G	28,000	7.2 34.9
	Linden Street	208	AW-2-6	1003	180	0	1	4	9		В	D		10.2 16.4
	Linden Street	218	RW-2-6	1008	160	0	0	0	4		F	B		0.0 9.2
	Cemetery Office Off Wahconah Str	eet	CW-2-0	1003	180	0	1	0	1	20	F	E		0.0 2.0
	Cemetery Chapel Off Wahconah Stre	eet	CB-2-0	1001	220	0	3	0	3	21	F	E		0.0 3.0
	Cemetery Garage Off Wahconah Stre	eet	CB-2-0	1001	210	0	3	0	3	22	F	Е		0.0 0.0
-	Body Shop Park Street	52	CB-1-0	1001	290	0	3	0	3	23	F	E	45,000	0.0 54.0

W. Branch					T	ABLE	İ					RE	ACH NO. 4
STREET	HOUSE	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER	DEPT WA' ABOVE	TER	1	TH OF DATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	COST IN \$ 1000,00
			ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR \$.P.
Park Street	53	RW-2-2	1004	110	0	0	3	8		В	В		6.6_ 6
Haddad Rug Compa		<u> </u>				_							
Park Street	56	CB-2-6	1001	360	0	3	6		· 24	В	E		20.4 19
Park Street	57	RW-2-6	1000	140	0	4	7	12		В	G	\$ 25,000	8.1 28
Park Street	77	CW-1-0	994	380	5	10	5	10		G	G	84,000	92.4 92
Seymour Street	112	AW-2-2	1012	N/A	0	0	0	0	*	F	F		0.0 0
Friendship Bar													
Seymour Street		CW-2-6	1006	290	0	0_	1_1_	6		B	В		16.4 16
Seymour Street	118- 120	AW-2-6	1008	190	0	0	0	4		F	В		0.0 10
Seymour Street	121- 123	AW-2-6	1006	184	0	0	1	6		В	В		10.4 10
Seymour Street	125- 127	AW-2-6	1005	194	0	0	2	7		В	В		11.0 11
Church of the Ho											ļ		
Seymour Street		CB-2-2	1003	240	0	1	6	11	25	В	E		13.5 30
Church of the Ho Seymour Street		mily CB-1-2	1010	300	0	0	0	4	25	F	E		0.0 75
Boston Fish Marke Seymour Street		CB-2-6	1001	350	0	3	6	11	26	В	E		19.8 24
Richmond Bakery Seymour Street		CB-2-0	999	380	0	5	0	5		F	G	90,000	0.0 112
								-					
Turner Avenue	9-	AW-2-6*	1006	180	0	0	1	6		В	В		10.2 10

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	W. Branch						T	ABLE	1					R	EACH NO.	4
	STREET	1	łouse No	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER		H OF IER E F.F	1	TH OF PATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET	\$ 1000	T IN D.00'S
					ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S,P.F.	VALUE	100 YR	S.P.F.
	Turner Ave	nue	10	RW-2-2	1006	160	0	0	1	6		В	В	-	9.2	9.2
j	Turner Ave	nue	12	RW-2-2	1003	170	0	1.	4	9		В	D	:	9.6	16.0
	Turner Ave	4	20 21 -	RW-1-1	996	190	3	8	6	11		D	G	\$ 34,000	16.7	37.4
ļ	Turner Ave			AW-2½-6*	996	190	3	8	1.1	16		D	G	13,000	16.7	20.1
	Turner Ave		25- 27	AW-2³₂-6*	996	190	3	8	11	16		D	G	13,000	16.7	20.1
-39-	_Turner Ave	nue	31	RB-2-0	994	180	5	10	 5	10		G	G	20,000	25.1	25.1
' 	Turner Ave	nue	64	RW-2-2	999	140	0	5	8	13		В	G	25,000	8.1	28.1
	Turner Ave	nue	72	RW-2-1	998	100	1_	6	4_	9		D	G	17,000	13.0	18.6
	Turner Ave	nue	76	RW-2-1	998	100	1.	6	4	9		D	G	17,000	13.0	18.6
	Turner Ave	nue	82	RW-2-1	998	100	1	6	4	9		D	G	17,000	13.0	18.6
	Turner Ave	nue	86	RW-2-1	998	100	1	6	4	9		D	G	17,000	13.0	18.6
	Turner Ave	nue	89	RW-2-6	999	160	0	5	8	13		В	G	32,000	9.2	36.0
	Turner Ave	nue	90	RW-2-1	998	110	1	6	4	9		D	G	15,000	13.4	16.9
	Turner Ave	nue	91	RW-2-2	999	130	0	5	8	13		В	G	21,000	7.6	23.6
	Turner Ave	nue	92	RW-1½-6	998	130	1	6	9	14		Đ	G	21,000	14.3	22.5
	Turner Ave	nue	93	RW-2-2	999	130	0	5	88	13		В	G	21,000	7.6	23.6

	W. Branch			<u>-</u>		T	ABLE	I			, ,-	- 1	REA	CH NO.	4
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER	DEPT WAT ABOVE	rer	DEPT INUND	H OF ATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET	COST \$ 1000	
1		"		ELEVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VALUE	100 YR	S.P.F.
	Turner Avenue	94	RW-2-6	998	170	1	6	9	14		D	G	\$ 27,000	16.0	31.5
	Turner Avenue	95	RW-2-2	999	160	0	5	_8	13		В	G	24,000	9.2	28.0
ļ	Church Wahconah Street	76	CW-2-2	1008	143	0	0	0	4		F	В		0.0	8.3
	Wahconah Street	82	RW-2-2	1003	160	0	1	4	9		В	D		9.2	15,6
-40-	Dick's Variery Wahconah Street	85	CW-2-6	1002	180	0	2	 5	10		В	D		10.2	16.4
Ť	Wahconah Street Jerry's Cafe	88	RW-2-2	1004	150	0	0	3	8		В	В		8,6	8.6
	Wahconah Street	89	CB-1-0	1001	190	0	3	0	3	27	F	E	19,000	0.0	22.4
	Wahconah Street	93 95- 99	AW-2-6	1005	230	0	0	2	7		В	В		13.0	
	Wahconah Street		CB-2-0	1003	230	0	1	0_	11	28	F	E		0.0	4_0
ļ	Wahconah Street	115~ 123	CB-1-0	1000	340	0	4	0	4		F	G	72,000	0.0	82.8
	Vale Florists - (Wahconah Street		house Att	ached 1001	390	0	3	0	3	29	F	E		0,0	8,0
	Wahconah Street	126~ 128	AW-2-2	1004	210	0	0	3	8		В	В		11.8	11.8
	Wahconah Street		CW-1-0	999	320	, O	5	0	5_			G	64,000	0.0	73.6
	Wahconah Street	132- 136	AW-2-6	1004	250	0	0	3	8		В	В		14.1	14.1
	Stadium Wahconah Street		CM-1-0	994	N/A	5	10	5	10	30		E		0.0	0.0

	W. Branch					Ţ	ABLE	I					REA	CH NO. 4	
	STREET	HOUSE NO	TYPE	ESTIMATED F.F. ELEVATION	ESTIMATED PERIMETER		TH OF TER E F.F	ı	H OF ATION	FOOTNOTE #	FLOOD	POSED PROOF INIQUE	ESTIMATED MARKET VALUE	COST \$ 1000.	
				CECVATION	LF	100 YR	S.P.F.	100 YR	S.P.F.		100 YR	S.P.F.	VACUE	100 YR	\$.P.F.
	Wahconah Street	139	RW-2-6	1002	120	0	2	5	1.0		В	D		7.1	13.8
1	LaCocina Restaura	1 1	l .											ļ	İ
1	Wahconah Street	140	AW-3-0	1003	160	0	1_	0_	1	31	F	<u>E</u>		0.0	2.0
	Wahconah Street	144- 146	AW-2-6	1008	210	0	0	0	4		F	В		0.0	11.8
1	Adrien's Diner						_		_					1	
-	Wahconah Street	145	CB-1-0	1003	170	0	1	0	1	32	F	E		0.0	14.0
	Wahconah Street	150- 152	AW-2-6	1008	190	0	0	0	4		F	В		0.0	10.7
	Wahconah Street	151	RW-2-2	1005	190	0	0	2	7		В	В		10.7	10.7
-41-	Wahconah Street	153- 155	AW-2-6	1005	170	0	0_	2	7		В	В		9.6	9.6
	Wahconah Street	157	RW-2-6	1006	200	0	0	1	6		В	В		11.3	11.3
İ	(Attached to 157)	۱ ا													
	Wahconah Street	161	CB-1-0	1006	N/A	0	0_	0_	0	*	F_	F		0.0	0.0
	Wahconah Street	163	RW-2-6	1006	140	0	0	1	6		В	В		8.1	8.1
	Wahconah Street	169	CW-1-0	1004	N/A	0_	0	0	0	*	_ F	F		0.0	0.0
	Wahconah Street	169R	RW-2-2	1000	100	0	4	7	12		В	G	\$ 25,000	6.2	26.6
	Closed Gas Static Wahconah Street	on 180	CB-1-0	1004	N/A	0	0	0	0	*	F	F		0.0	0.0
	Shea's Rugs Wahconah Street		CW-1-0	1007	N/A	0	0	0	0	*	F	F		0.0	0.0
	Wahconah Street	187	AB-2-6	1002	200	0	2	5	10	33	В	E		11.3	3.0
	Liquor Store, sam Wahconah Street			s Nichols 1006	Pharmacy N/A	0	0	0	0	*	F	F		0.0	0.0

W. Branch					T	ABLE	I					R	EACH NO.	4
STREET	HOUSE	ТҮРЕ	ESTIMATED F.F.	ESTIMATED PERIMETER	DEPT WAT ABOVE	ER	1	TH OF ATION	FOOTNOTE #	FLOOD	POSED PROOF HNIQUE	ESTIMATED MARKET	COST \$ 1000	
	"	<u> </u>	ELEVATION	LF	IOO YR	S.P.F.	IOO YR	S.P.F.	 	100 YR	S.P.F.	VALUE	100 YR	\$.P.F.
Nichols Pharmacy Wahconah Street		CB~2-0	1006	N/A	0	0	0	0	*	F	F		0.0	0.0
Wahconah Street	210- 218	AW-2-6	1003	320	00	1	4	9		В	D		18.0	24.0
Wahconah Street	224	AW-2-6	1003	240	0	. 1	4	9		В	D		13.5	18.0
Wahconah Street	224R	AW-2-6	1003	200	0	11	4	9		В	D		11.3	17.1
Wahconah Street	225	RW-2-6	1003	200	0	1	4	9		В	D		11.3	17.1
Wahconah Street	225R	RW~2-6	1003	200	0	1_	4	9		В			11.3	17.1
Wahconah Street	234	CW-2-6*	1004	220	0_	0	3_	8		В	В	<u> </u>	12.4	12.4
Wahconah Street	235	RW-2-0	1004	N/A	0	0	0	0	*	F	F		0.0	0.0
Wahconah Street	237	AW-2-6	1005	160	0	0	2	77		В	В	-	9.2	9.2
Wahconah Street	239	CB-2-0	1005	N/A	0	0	0	0	*	F	F		0.0	0.0
Wahconah Street	240	RW-2 ¹ 2-6	1007	200	0	0	0	5		F	В		0.0	11.3
Wahconah Street	242 - 244	AW-2-6	1006	140	0	0	1	6		В	В	<u> </u>	8.1	8.1
Wahconah Street	248	AW-2-6	1005	160	0	0	2	7		В	В		9.2	9.2
Wahconah Street	252	CB-1-0	1005	N/A	0	0	0	0	*	F	F		0.0	0.0
Wahconah Street	256	RW-2-6	1006	170	0	0	1.	6		В_	B		9.6	9.6
Apt. Complex Wahconah Street		AW-2-0	1006	N/A	0	0	0	0	*	F	F		0.0	0.0

,

ţ

TABLE II

	_							SIZE	IN	PERIM	ETER	FEET					F.	LOODE	ROOF	ING M	ETHO)				
		3	STRUC. DIED	CES	IAL	NTS	IAL	9	20	.70	71			A		В	. (C		D .]	E		F	(G
•		REACH	# OF STI STUDIE	# OF RESIDENCES	# OF COMMERCIAL	# OF APARTMENTS	# OF INDUSTRIAL	0 – 76	77 - 1.	125 - 1	OVER 1	N/A	100 YR	SPF	100 YR	SPF	100 YR	SPF	100 YR	SPF	100 YR	SPF	100 YR	SPF	100 YR	SPF
	BRANCH	1	1	1	0	0	0	ó	0	1	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0
	- [2	_71	54	13	_4_	<u>o</u> .	_1	<u> 36</u>	24	10	0	1_1_	_0	10	_8_	_8_	_3_	_7_	4_	11	_11	9	0	25	45
	N. W.		72	55	13	4	0	1	36	25	10	0	1	0	10	8	8	3	7	5	11	11	10	0	25	45
23		1	7	0	1	6	0	0	0	0	0	7	0	0	0	0	0	0	0	0	0	0	7	7	0	0
	H	2	10	8	1	1.	0	0	3	6	1	0	0	0	4	2	2	4	0	2	0	1	3	0	1	1
	BRANCH	3	100	62	11	27	0	0	13	49	31	7	9	2	44	28	1	1	19	34	2	4	23	7	2	24
	WEST	4	178	107	33	38	0	_0	27	74	55	22	_2	4	87	<u>55</u>	10	_7_	14	33	_3_	18	60	21_	_2	40
	W		295	177	46	72	0	0	43	129	87	36	11	6	135	85	13	12	33	69	5	23	93	35	5	65
	TO	TALS	367	232	59	76	0	1	79	154	97	36	12	6	145	93	21	1.5	40	74	16	34	103	35	30	110

TABLE III
FLOODPROOFING COSTS

		FLOO	DPROOFING TECHN:	QUE	<u> </u>	·	
(PERIMETER FT.) SIZE	(DOLLARS A	PER B	PERIMETER FT.) C	(LUMP S	SUM COST IN E	THOUSAND F	DOLLARS) G
0 - 76	37	64	71	12	Indi	0	Indi f
77 - 124	35	59	63	14	ividu for e	0	vid
125 - 170	34	57	61	16	lual c	0	dual c each
≥ 171	35	56	61	18	Jase	0	ase

TABLE IV

FLOODPROOFING COST ACCORDING TO REACH

	REACH NUMBER	NUMBER OF		COST THOUSANDS (
H		100 YEAR	SPF	100 YEAR	SPF
BRANCH	1	0	ı	0.0	15.6
S.W.	2	62	71	5118,1	5816.7
	1	0	0	0.0	0.0
BRANCH	2	7	10	60.1	109.7
WEST I	3	77	. 93	1017.4	1599.3
is .	4	117	156	1290.8	2798.9
-	TOTALS	263	331	7486.4	10340.2

^{*} EXCLUDING THE STADIUM OFF WAHCONAH STREET (NO COST INVOLVED)

TABLE V
FLOODPROOFING COSTS ACCORDING TO CATEGORY

	# OF CA	SES	COST IN	THOUSANDS	AVERAGE CASE IN T	COST PER THOUSANDS
CATEGORY	100 YEAR	SPF	100 YEAR	SPF	100 YEAR	SPF
RESIDENTIAL	181	218	2306.7	3932.0	12.74	18.04
COMMERCIAL	29	48	4569.6	5361.0	157.57	111.69
APARTMENT	53	65	610.1	1047.2	11.51	16.11
INDUSTRIAL			0.0	0.0	0.0	0.0
TOTALS	263	331	7486.4	10340.2	28.47	31.17

III. CONCLUSIONS

Included within Table IV is a summary of floodproofing costs for the entire study area according to Branch and Reach Number. This report estimates a project cost of approximately 7.5 million dollars (\$7,500,000) for the 100 Year Flood levels and 10.3 million dollars (\$10,300,000) for the Standard Project Flood levels.

A review of Table V indicates that residential property constitutes the largest portion of the total project cost impact for both flood levels studied. However, the financial impact to the commercial properties (and to the City of Pittsfield) as shown in Table V is much more difficult to estimate as part of the true cost of non-structural flood damage prevention. This report did not attempt to estimate the costs of acquiring businesses for structures found to be impossible to floodproof and therefore assumed to require demolition; the cost and availability of commercial property to relocate those businesses; or the practicality of relocating a commercial business with its viability dependent upon location.

In reviewing Table I, Table II and Appendix B (pages 33 - 38), the impact of the demolition category upon the total project estimate can be analyzed. Of the 72 structures in the Southwest Branch, 35 structures under the 100 year flood and 55 under the SPF are categorized as requiring demolition. This represents 49 and 76 percent of all structures in this branch respectively. However, from the project cost

standpoint, under the 100 year flood, \$4,860,300 (95%) of the total estimated floodproofing cost of \$5,118,100 for the Southwest Branch is attributable to demolition; and under the SPF, \$5,650,500 (97%) of the total amount of \$5,816,700 is attributable to demolition. Should a project of this scope be implemented along the Southwest Branch of the Housatonic River, the impact of demolition upon the community in this area would be extremely significant, both physically and financially.

In the West Branch area surveyed for this report, the impact of demolition upon the total estimated floodproofing cost is not as significant. Of the 295 structures in the West Branch, only 9 under the 100 year flood and 71 under the SPF (3% and 24% of all structures in this Branch respectively) fall under the demolition category. From an estimated floodproofing cost standpoint, \$422,800 (18%) of the \$2,368,300 total estimated floodproofing cost for this Branch under the 100 year flood and \$2,160,400 (48%) of the total \$4,507,900 under the SPF is attributable to demolition. This too, represents a significant impact, but not to the extent seen in the Southwest Branch area.

Several major estimates and assumptions were made within this study which may have a large effect upon the actual final project cost should such a project be instituted. The following represents some of these estimations and assumptions made during this study:

- A. The study areas of this report are limited solely to the areas delineated for the 100 year and the SPF flood levels as shown on Sheets 1-4 in the Attachments.
- B. It was assumed that wooden structures being inundated above the first floor elevation from one to three feet could be raised (this could only be verified after an in-depth structural analysis of the building). Structures falling in this category would be floodproofed using Technique C in combination with sealing the doors and windows in the foundation wall. This assumption may not be in accordance with local fire codes or the owner's wishes. Alternate floodproofing techniques may prove more expensive than the above method.

Structures, primarily commercial, being inundated above the first floor elevation from one to three feet were reviewed on a case by case basis. It is impractical to assume that a masonry structure, generally constructed on a slab, could be raised. Therefore, each structure was reviewed to see if an alternate method of floodproofing would be possible, such as placing shields over windows or construction of earth berms. If no practical solutions were apparent, an estimated cost for demolition of the structure was made and entered in Table I.

C. Structures which had a depth of inundation in excess of three feet above the first floor were assumed to require demolition. Further analysis of the structures and additional cost studies would be required to evaluate if the structure could physically be relocated and if vacant property would be available.

- D. A factor which affects a large number of structures within this study is the item involving the waterproofing of the outside of basement walls. For this investigation, it was assumed that all structures in fair to excellent condition have finished basements with storage. In many cases, the basement walls may actually be unfinished rough concrete. A savings may be seen by the elimination of the outside trench should it be determined that such structures could be waterproofed from the inside. It may also be found that the proposed trench excavation may not be possible without affecting the structural integrity of the building. This could be due to the nature of the material making up the foundation, the overall condition of the foundation or the layout of the foundation which may prevent access. Should such a situation occur, the final solution may cost much more than the estimated cost herein.
- E. Another factor which could have the largest effect upon the success of the non-structural project is the cooperation of the people who would be affected. Should these people offer little or no cooperation, the projected implementation time would be increased and new solutions may have to be sought. Such actions would change the project costs significantly.
- F. Although costs have been estimated in Table I for the fair market value of apartments falling in the demolition category, no attempt was made to estimate the subsequent costs of relocating the tenants of those apartments to new living quarters. Such costs are difficult to estimate

and would result in an actual cost for apartments requiring demolition somewhat higher than presented in the scope of this report.

APPENDIX A

APPENDIX A

* (NOTE: Structures classified under this footnote were included in the field survey as appearing to fall within the flood plain limits, but field reconnaissance showed them to be above SPF inundation. They appear herein only to show that consideration was given to them.

SOUTHWEST BRANCH

1. 112 Jackson Street - Radio Station WBEC

This modern building is a one story concrete block office building and radio station constructed as a slab-on-grade with a drainage ditch running parallel to the rear of the building.

At the 100 year storm, this structure would be subject to two feet of inundation and an additional seven feet under the SPF.

It is proposed, for purposes of this study, to provide a dike around the station itself, but not the towers. Storm drainage contained within the area enclosed by the dike could be removed by collecting and pumping the drainage to the exterior of the dike limits. Vehicular access would be required up and over the dike to the radio towers. The cost of this work is estimated to be \$21,000 for the 100 year storm and \$32,000 for the SPF.

2. Big "N" Mall Complex - W. Housatonic Street

This complex is a slab-on-grade, concrete block structure, with a glass front. Under the 100 year storm, it would be inundated by eight feet, and under the SPF, an additional seven feet.

Consideration was given to providing a dike around the area, however, it is our opinion that a dike would render this complex economically undesirable to any potential tenant. It is presently occupied to less than twenty percent of its total floor area. Since it is physically impossible to floodproof or raise this complex, it is included for demolition.

3. MacDonald's Hamburger Stand - W. Housatonic Street

This structure is a new, standard structure normal for this franchise. It is constructed within the present flood plain. Under the 100 year storm, it would be inundated by eleven feet of water, and under the SPF, by an additional seven feet.

For purposes of this study, this complex is included for demolition.

4. Fitch's Complex - 472-484 W. Housatonic Street

This complex consists of a twenty-two unit motel, owner's home at 472 W. Housatonic Street, Shell Gas Station and repair shop at 484 W. Housatonic Street, and a diner which is not numbered. All are subject to flooding of eleven feet under the 100 year flood and an additional seven feet under the SPF.

Since it is physically impossible to floodproof or raise these structures, this complex is included for demolition.

5. Sunoco Station - W. Housatonic Street

This structure is slab-on-grade, concrete block building with a seven bay general repair garage being an integral part of the business. Under the 100 year flood, it would be inundated by twelve feet of water, and by the SPF, an additional seven feet.

For purposes of this study, this building is included for demolition.

6. 581 W. Housatonic Street - Mobil Gas Station

This structure is of concrete block construction with a two bay garage as an integral part of the structure.

Under the 100 year storm, it would be inundated by five feet, and by an additional seven feet under the SPF. Since it is physically impossible to floodproof or raise this structure, it is included in the demolition category.

7. 607 W. Housatonic Street - Body Shop

This structure consists of three combined buildings which are of slab-on-grade and concrete block construction.

Inundations and recommendations are as under Footnote 6 above.

8. 615 W. Housatonic Street - Tire Shop

This structure is a slab-on-grade, concrete block structure with a four bay garage.

Under the 100 year storm, it would be inundated by four feet, and under the SPF, an additional seven feet. Since it is physically impossible to floodproof or raise this structure, it is included for demolition.

WEST BRANCH

9. 301 Columbus Avenue - Riverside Cafe

This two story brick structure situated adjacent to the Housatonic River would be inundated by four feet under the 100 year flood, and by an additional six feet under the SPF. The SPF flood would inundate the first floor of this structure by two feet.

Due to the condition of this structure's foundation and the physical characteristics of the building, waterproofing the basement and raising the foundation are not feasible. Thus, this structure is included in the demolition category under the SPF.

10. 300 Columbus Avenue - West Side Fish Market

This one story commercial structure is constructed of concrete block on a slab-on-grade and would be subject to three feet of inundation under the SPF.

Due to the physical characteristics of this building, raising the foundation is not feasible. Thus, for the purposes of this study, it is included in the demolition category for the SPF.

11. 97 Dewey Avenue - Milton's Place

This structure is concrete block, slab-on-grade. At both 100 year storm and SPF, this structure would experience flooding. Due to its construction, raising this structure is not feasible. Thus, for purposes of this study, it is included under demolition.

12. 154 Dewey Avenue

This building is a two story brick building on a slab-on-grade and would be subject to three feet of inundation at SPF. Since it is physically impossible to raise this structure without damage, it would be necessary to install flood shields along the front and to apply waterproofing to the level of the SPF. This work would cost approximately \$5,000.

13. 175 Dewey Avenue

The major portion of this two story residence is constructed of concrete block, but the rear section is wood frame slab-on-grade. Under the SPF, this structure would be subject to two feet of inundation.

Due to the size and characteristics of this structure, raising its foundation is not feasible. Thus, for purposes of this study, it is included under demolition for the SPF.

14. 198 Linden Street - Amoco Gas Station

This concrete block, slab-on-grade, two bay and office service station is subject to flooding at both the 100 year flood and the SPF. Due to its construction, raising the structure for the 100 year flood is not feasible. Therefore, demolition is included for both flood levels.

15. 50 Danforth Avenue

This home is constructed with brick facing and fieldstone around its entry on a slab-on-grade.

At SPF this structure would be subject to three feet of inundation. As this building would be severely damaged by any attempt to raise it, and demolition is impractical for purposes of this study, this residence would be provided with removable flashboards at the two doors and water-proofing to the underside of windows.

The cost of the flood shields is estimated at \$1,100.

16. 82 Danforth Avenue

This structure is a two story wood frame residence constructed on a slab-on-grade. Under the 100 year storm it would be subject to three feet of inundation, and an additional five feet under the SPF.

Due to the construction of the building, raising the foundation is not feasible. Therefore, for the purposes of this study, it is included in the demolition category under both the 100 year storm and the SPF.

17. 39 Elm Vale Place - Fahey Beverage Company

This new steel panel clad, one story warehouse type structure has a slab-on-grade with a two bay truck dock being an integral part of the building. The truck approach area is four feet below finished floor.

This facility is subject to theoretical flooding under the SPF. For purposes of this study, we include the cost of raising the structure above this level and the reconstruction of the base slab to accommodate this raised structure.

The estimated cost for this work is \$110,000.

18. Elm Vale Place - J.B. Paper Company

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This complex consists of five buildings, four of which are interconnected and constitute the smaller of the two structures shown on the plan. All structures are brick constructed on a slab-on-grade and would be subject to flooding under the SPF. On the west side of the building is a three bay and a two bay truck dock as well as miscellaneous access doors. Steel casement windows are three feet above finished floor.

Protecting these structures against flooding would require the installation of flashboards at all doors and the floodproofing of all exterior walls up to the underside of the windows.

The estimated cost for the above flood protection is \$16,000.

19. Linden Street - Bob's Automotive

This structure is a one story commercial building constructed of concrete block on a slab-on-grade. Under the 100 year storm it would be subject to two feet of inundation, and subject to an additional five feet of inundation under the SPF.

Since it is physically impossible to raise this structure without causing damage, it is included in the demolition category under the 100 year storm.

20. Off Wahconah Street - Cemetery Office

This two story building is constructed of brick, and set on a slab-on-grade. It would be subject to one foot of inundation under the SPF.

Due to the excellent state of repair and usage of this structure, it is placed in the category of being floodproofed by the use of waterproofing and flood shields.

The cost of this work is estimated at \$2,000.

21. Off Wahconah Street - Cemetery Chapel

This structure is constructed of granite blocks set on a slab-ongrade. It would be subject to three feet of inundation under the SPF.

Due to the excellent condition and nature of this structure, it is included in the waterproofing and flood shield category.

The cost of the floodproofing work is estimated at \$3,000.

22. Off Wahconah Street - Cemetery Garage

This is a two story brick structure constructed on a slab-on-grade. It would be subject to three feet of inundation under the SPF.

Due to the nature of this structure, it is included in the category of requiring no formal technique of floodproofing, since floodwaters could enter and recede from the garage while causing minimal damage.

23. 52 Park Street - Keene Body Shop

This building is made up of a brick portion and two distinct buildings of wood, all being interconnected. All are a slab-on-grade.

Under the SPF, this structure would be subject to three feet of inundation. Since it is physically impossible to raise these buildings or to provide a dike, for purposes of this study this complex is included for demolition.

24. 56 Park Street - Haddad Rug Company

This building, with basement, is a two story brick structure which would be subject to three feet of inundation above the first floor under the SPF. The basement, for both storms, would be subject to flooding. The floodproofing for this structure would be to install removable flood shields at the entrances and the basement windows, and to utilize the floodproofing technique presented in Table I.

The cost for the flood shields is estimated at \$19,000.

25. 133 Seymour Street - Church of the Holy Family & Rectory

The church and attached rectory are one structure of substantial brick construction on a concrete foundation with differing first floor elevations. The basement of the rectory would be subject to flooding under both the 100 year storm and SPF, but the church itself would be subject to inundation of the basement only under the SPF.

Due to the size, physical characteristics and nature of the structure, floodproofing measures considered include an interior drain and sump pump system and removable flood shields covering all openings to one foot above the SPF level.

The estimated cost of these floodproofing measures is approximately \$105,000; not including costs for backflow prevention through building drains and sanitary waste lines.

26. 159 Seymour Street - Boston Fish Market

This building is a two story brick building in excellent condition. Since it is physically impossible to raise this structure without damage, it would be necessary to install flood shields at all entrances and to apply waterproofing to the level of the SPF. Cellar windows would be sealed with brick.

This work would cost approximately \$24,000.

27. 89 Wahconah Street - Jerry's Cafe

This is a two-story wood frame structure with an addition constructed of brick on a slab-on-grade which fronts the street. It would be subject to three feet of inundation under the SPF.

Since it is physically impossible to raise the foundation of this structure or floodproof the building without impairing its function, it is included in the demolition category under the SPF.

28. 105-109 Wahconah Street

This group of buildings is of concrete block construction set on a slab-on-grade. It would be subject to one foot of inundation under the SPF.

Floodproofing this structure would entail the application of waterproofing to the lower two feet of the exterior perimeter of the building and the installation of removable flood shields at all entrances.

The cost of this work is estimated at \$4,000.

29. 120 Wahconah Street - Vale Florists

This complex consists of six greenhouses, a connecting shed, and a sales and office building. The sales and office building is a slab-on-grade, one story wooden structure which could be raised, therefore costs have been included for raising this structure only, leaving the greenhouses to flood should a SPF occur. The greenhouses would be subject to three feet of inundation under the SPF.

The cost of this work is estimated at \$8,000.

30. Wahconah Street - Stadium

The structure of concern at this sports stadium is a one story metal-clad, slab-on-grade building at the sports field on the nearer side to Wahconah Street. It would be subject to five feet of inundation under the 100 year storm and ten feet under the SPF.

Due to the construction and function of this structure, it is included in the category of requiring no method of floodproofing considered in this report.

31. 140 Wahconah Street - La Cocina Restaurant (1st floor)

This structure is a three story wood frame apartment constructed on a slab-on-grade. It would be subject to one foot of inundation under the SPF.

Floodproofing this structure may be accomplished by the installation of an exterior-mounted steel flood shield on the building perimeter with demountable flood shields at all entrances.

The cost of these measures is estimated at \$2,000.

32. 145 Wahconah Street - Adrien's Diner

The front portion of this building is a dining car, the attached portion being of concrete block on an elevated slab-on-grade. It would be subject to one foot of inundation under the SPF.

The dining car portion could be raised to protect this structure against the SPF and the rear portion protected with removable flood shields at entrances and waterproofing applied to the lower two feet of exterior walls.

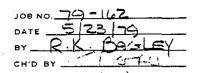
The cost of this work, including modifications to the interface between the two structures, is estimated at \$14,000.

33. 187 Wahconah Street

This structure is a two story brick apartment building in relatively good condition. It would be subject to two feet of inundation under the SPF. As this structure could not be raised without damage to its integrity, floodproofing measures would include providing floodproofing sealing basement windows and access, and the installation of flash-boards as well as the waterproofing to the underside of the windows.

The cost of this work is estimated at \$3,000.

APPENDIX B

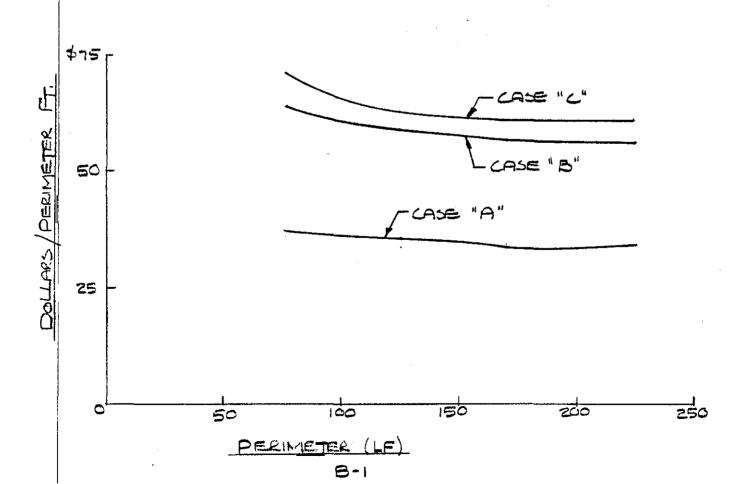




JOB PITTELO
SUBJECT FLOCO STUDY
CLIENT C.O.E.

cost factor summary

		FL	FLOOPROOFING CATEGORY		
House Size	PERIMETER	المحا	LARS/PERIM		LUMBOM
(LENGTH & WIDTH)	(LINEAR FEET)	A	් ය	ر	۵
16' x 20'	76	36,95	64.37	71.28	12,000
'ما3 × 'مل2	124	35,27	58. ⁸³	63 ,∞	14,000
35' > 50'	170	34,06	56,60	60.70	مص ما
50'x 62'	224	34,66	56,35	60.03	18,000.



JOE NO	79-162 .1	_
DATE	5/21/79	
BY	K' Bacley	_
SH'D BY	J RISTALL	

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON: MASSACHUSETTS

JOB PITTS FIELD

SUBJECT FLOOR PROFING

CLIENT COE

(Wage Rates From "Means"-include O.H. & P.)

I. FLOODPROOFING - 26'x 26' HOME
(Perim = 124 feet)

A. DRAINS & SUMP PUMP

1. Breaking up alab around cellar floor

Rent air compressor @ \$50/day x zdays \$100.

1 worker for zdays: 16hrs. @ \$14.00/hr zat.

(Includes deenup) \$334.

2 Hand excevate trench, remove metil from cellar; load & haul away
Assume; 2 man craw 6 \$31.50/hr.

Trench zidp. x z'wd @ base,
1:2 sides

1/2 (4+2) 2 = 6 sq.ft. Inside perim. 2' from wall: 108' Volume = 648/27 = 24 cy.

Assume: 2 laborers candig 12 cylday
2 " move 24 cylday

2 days digging @ \$31.50/crewx8h7/day \$504.

1 day loading @ \$31.50/crewx8h7/day \$504.

Dump Truck (Locy) @ \$ 135/day Pay loacher: 4 hrs @ \$34/hr

135. <u>136.</u> \$ 1,027.

3. Backfill - crushed stone 24 cy. @ 46/cy. (mat'l) 5 hrs labor @ 414.10/hr

\$ 144. \$ 761

JOBN	ب	9-	16	2	1	_
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HH &B	HAYDEN, HARDING & BUCHANAN.	INC.
_	BOSTON MASSACHUSETTS	

~	GHE	E NO 3	136
JOB 1	ITSF	ELD	
SUBJECT -	=000	<u> </u>	
CLIENT			<u> </u>

590

4. 6" V.C.P.
100 1.f. @ \$4/1.f. (including installation) \$432.

5. Replacing Concrete
4' x 4/12 thk. x 108 l.f./27 =
5 /3 c.y. @ \$ 110/cy

6. Sump Pump, Hose, Install Gutlet 275

7. Clean up, Replace tiles, etc.

5 hrs @ \$14.00/hr. 117.

TOTAL \$3,036.

B. WATERPROOF WALLS

1. Excessic Trench Arand House
Assume: 3.5' wd x u'dp.
Outside perim: 140' (z'from wall)
Volume: 3.5 x 6 x 140/27:
109 cy @ 4z.80/cy 4305.

2. Clean Walls, Apply Water proofing E' ht x 124' perm = 992 of. @ \$1./of. 992.

3. Backfill Trench w/ Compaction
109 cy @ \$3.60/c.y. 392.

4. Restore Site (Clean up, Replace Shrubs, Fences, etc.) 340

JOB NO.	79 - 1	WZ.	
DATE	5/21	170	
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HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON MASSACHUSETTS

JOB PHI SPIELD
SUBJECT FLOO ACTION
CLIENT C.O. E

15.

C. BLOCK UP WINDOWS

- 1. Remove Exist Windows Assume: 6 windows @ 2hrs labor/ea. 12 hrs. @ \$14,60/hr. \$175.
- 2. Material Concrete Blocks
 6 windows x 16" x 32"/ea / 144 =
 21 5.f. @ \$6,70/5.f
- 3. Installation
 Assume: | Bricklayer & \$18.55/hr

 1/2 hrs x 6 x \$18.55 = \$167

 Use Same As For Removal 175

 TOTAL \$365.

TOTAL PART IA : \$3,030.
"IB : 2,029
"IC : 365

TOTAL PART I; \$ 5,430.

JOB NO	75,-1	62.1
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JOB PITT SPIELD
SUBJECT Flood Hoofing
CLIENT C.O. E.

II. RAISING FOUNDATION

A. EXISTING CELLAR - 2 story, 24'x 34' ave home

1. Pipes e' Electric Lines

Heating, sewer, water, 925, electric,

telephone etc. lines must be cut e'

extended, junction boxes installed, etc.

L.S. Estimate \$1,700.

2 Underpinning - Jacking

Mobilization

Estimate to cut walls, underpin, jack

up, place cribbing, etc.

Labor: 3 days - 2 mechanics, 3 laborers

Mechanics: †21/hr x 3xBxZ

Laborers: †41/hrx 3xBxZ

Laborers: †41/hrx 3xBxZ

[05]

Equipment Rental For Duration

1,200.

3. Masonry
Pour or lay new foundation, fix all
stainways, fix or pour new floor slab.
Assume:

- 3' foundation x124' perm (new folm. walls) & \$3./5f. \$1,116.

- Stairweys & slab (Assume I mason & helper) \$33.25 (combined)/hr x Zdaysxs 532

\$1,648.

4. Materials
Lines, pipes, brick/block/concrete, etc.
Estimate L.S. +225.

5. Restoration
Clean up, shrubs, lawn, plaster cracked walls, restore basement, repair parches, etc.

1,150

6. Care for Occupants
Housing for duration, moving of
valuables, etc.

TOTAL PART II A: 10,632 TOTAL PART II A: 10,632 TOTAL PARTS IA IB, IC & IIA: \$10,062

<u>COSTS</u>

(See pp. 192 : IA)

Raw Cost : 10% Contrigencies:

\$3,036. (pg. 2) 304. 73,340.

General Contractor's OH&P UO%): 304.

Ergineering & Survey (20%): 729.

\$ 4,373,

COST / PERIM. FT. = \$4,373/124 = \$35.27/ft.

CASE "B"-DRAINAGE SYSTEM, SUMP PUMP & WATERPROOFING (See PG. 142, IIA & IIB)

PAW COST : 10% Contingencies: \$5,065 (pg.z).

507 \$ 5,572.

General Contractor's OH & P (10%): 507.

Engineering & Survey (20%): 1,216.

\$ 7,295.

COST / PERIM FT. = \$7,295/124 = \$58.83/ft.

JOB PITTOFIELD
SUBJECT FLOOR PROFING
CLIENT C.O.E.

CASE "C"- DRAINAGE SYSTEM, SUMP PUMP,

VLATER PROSPINCE & BLOCK UP WILLIAMS

(See Ap. 1-3: IA, IB & IC).

Raw Cost ; 10% Contingencies :

\$5,450, (25,5) 543. \$5,973.

General Contractor OH&P (10%):

\$ 6,516.

Engineering of Survey (20%):

1,303.

\$7,819,

COST / PERIM. FT. = \$7,819/124 = \$63,06/ft.

(See pp 4-5, IIA)

Raw Cost: 10% Contingencies:

10,632. (B.5) 1,063. 411,695.

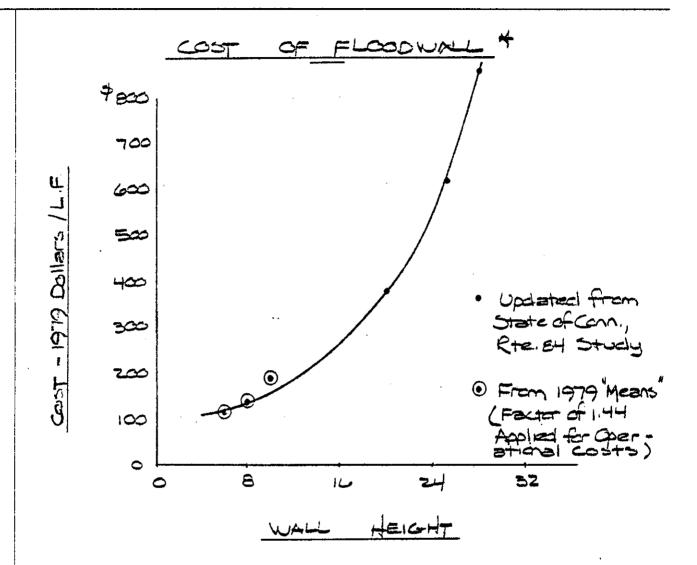
General Contractor OH&P(10%)

1,063. 312,758

Engineering of survey (10%)

1,276,

LUMP SUM - +14,034.



* Floodwall considered is reinforced concrete contilever, including all materials, installation is operational extra s.

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

JOB PRISELD
SUBJECT Floral Abofing
CLIENT C.O.E.

cost of flood shields

1. Material

Assume: 3'x3/z' gate, "4" thk. aluminum shield Volume: 3'x3/z'x 44/1z = 0.22 Cf Unit Wt: 1U5 pcf

Fotal weight (Assume 2 x wt. of gate for all brackets of fixtures)

0.22 cf x 165 pcf x 2 =

72# @ \$ 2.50/16.

\$ 150.

2 Installation

Assume: A crew of Imason of I helper can install Itz shields / day, or approx. Time to install I shield = 6 hrs.

6 hrs @ (\$16.55+14.60)/hr

199

Raw Total: \$374.
10% Contingencies: 39
Contractor's OHAP (10%): 38
\$455.
Engineering (20%): 91

\$546.

USE \$ 550 / Shield

JOB NO	70-1606.1	_
DATE	5 22 79	-
BY		_
CH'D BY .	J. VISTAL	-

HH HAYDEN, HARDING & BUCHANAN, INC.

SHEET NO 11 / MANN

JOB PITTS FIF L.D'

SUBJECT Flood Possing

CLIENT C.O.E.

(wage Rates from "Means" - Include G.H. & A).

I FLOODPROOFING - 18'x20' House (Perm = 76')

A DRAINS & SUMP PUMP

1. Breaking up alab around cellar floor

Rent air compressor @ \$50/day for Iday \$50.

1 Worker for Iday: \$14.00/hr x 8h7/day 117:

\$107.

2. Hand Excavate Trench, remove matil
from cellar, "load of haul away
Assume: 2 man work crew @ \$31.50/hr
Trench z'dp., z'wd. @ base,
1:2 sides

1/2 (4+2) 2 = 6 sq. ft. Inside perim z' from well: 60' Volume = 360/27 = 13 64.

Assume: 2 laborers can dig 12 cylday
z " move 24 cylday

1 day diging 6 \$31.50/hr x 6 hrs/day 12 day beding 6 \$31.50/hr x 6 hrs/day

\$252, 126,

Domp Truck (604) @ \$ 135/day Abyloader: 4 hrs @ \$34/hr

8-11

136 136 4649,

3. Backfill - crushed stone
14 cy @ 46/cy (matil)
6 hrs labor@ 414.00/hr

*84 <u>85</u> †172,

JOB NO. 719-162.1
DATE 5/22:79
ay R.K. Booley
CH'D BY

HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON: MASSACHUSETTS

JOB PIT SFIELD	₹,
BUBJECT Flood Proofin	<u>5</u>
CLIENT	

4. 6" V.C.P 601f @ 4/1f. (Including Installation) + 240. 5 Replacing Concrete 41x 8/12 x 60 1f /27 = 3 cy 6 \$110/c4 330 6. Sump Pump, Hose, Install Outlet 275 7. Clean up , Replace tiles , etc. 6 hrs @ \$14.00) hr TOTAL-B. WATERPROOF WALLS 1. Excepte Trench Around House Assume : 3,5'ust xu'do, Outside perim : 100 (2/ from wall).
Volume - 3.5 x6 x100 /27 = 78 Cy @ \$ 2 80/cy \$ 218, 2. Clean walls , Apply water proofing 8' ht. x 70' perim = 608 s.f. 6 41/s.f. රථා 3. Backfill Trench w/ Compaction 70 cy. @ \$3.00/cy 281

H. Restore Site (Clean up, Reptace
Shrulos, Fences, etc.)

TOTAL

\$ 1,447.

TOTAL IA & IB

\$ 3,397.

B-12

JOB PITTSFIELD
SUBJECT FLOOR PROFING
CLIENT C.O.E.

C. BLOCK UP WINDOWS

- 1. Remove Exist. Windows
 Assume: 6 windows @ Z hrs./abor/ea.
 12 hrs. @ \$ 14,60/hr. \$ 17
- 2. Material Concrete Blocks 6 windows x 10"x32"/ea/144 = 21 s.f. @ \$0,70/s.f. 15.
- 3. Installation
 Assume: I Bricklayer & \$18.55/hr.
 1/2/no.xux \$18.55/hr = \$167.
 Use Same As For Removal

TOTAL PART IA; \$1,950.

TOTAL ART I: \$3,762.

JOB PITSFIELD

SUBJECT FLOW POSTICE

CLIENT C.G.E.

II. RAISING FOUNDATION

A EXISTING CELLAR

- Disconnect & Restore Lines
 Hesting, sewer, water, gas, electric,
 telephone etc. lines must be cut &
 extended, junction boxes installed, etc.
 L.S. Estimate
 41,700
- 3. Maxing

 Pour or lay new foundation fix all

 Stainways, fix or pour new floor slabs

 Assume:

 -3' foundation x 76' perim (new fdn.

 walls) @ \$3/sf

 Stainways & slab (Assume

 1 maxin & helper)

 \$25,25 (combined)/hr x1/2 days x 8 399.

 \$1,083.

JOB PITTOFIELD
SUBJECT FLOOD FORTING
CLIENT CO F

4. Materials
Lines, pipes, brick/block/concrete, etc.
Estimate L.S. \$225.

5 Restaration
Clean up, shrubs, lawn, plaster cracked walls, restore basement, repair parches, etc. .850

U. Care for Gupants
Hasing for duration, moving of
Valuables, etc.
Assume:
30 day duration 6 445 play 1,950

TOTAL +9,081,

TOTAL PART IA: 9,081

TOTAL ARTS JAFE IL GIA: \$ 12,843.

JOB PITTOFIC BOLLED

SUBJECT FIRM AROTHER

CLIENT C.O.E.

COSTS

CASE "A" - DRAINAGE SYSTEM WITH SUMP PUMP (SEC PP. 10 4 11, IA).

Raw Cost:

10% Contingencies:

General Contractor Ottep (10%):

Engineering & Durvey (20%)

+ 2,808.

COST / PERIM. FT = \$2,808/74 = \$34.95/Ft.

CASE 'B"- DRAHAGE SYSTEM, SUMP PUMP & WATERPROOFING (See pp. 10 & 11, IA & FB)

Raw Cost:

10% Contingencies:

General Contractor OHEP (10%):

540.

\$41,077.

Engineering & Survey (20%)

\$1,892.

COST / PERIM . FT. = \$4,892/16 = \$64,57/ft.

JOB PITTOF IF LO SUBJECT FLOOD PROFING

CASE "C"- DRAINAGE SYSTEM, SUMP PUMP,

WATER PROOFING & BLOCK UP WINDOWS

(See pp. 10-12; IA, IB & IC)

Raw Cost:

10% Contingencies:

General Contractor OHEP (10%)

576.

54,514.

Engineering of Survey (20%):

903.

\$5,417.

COST / PERIM: FT. = \$5,417./76 = \$71,28/ft.

CASE "D" - RAISE HOUSE 3 FEET (SEC AD. 13-14, IIA).

Raw (057: \$9,081.
10% Contingencies: 906.
General Contractor OH&P (10%): 908
\$10,897.
Engineering & Survey (10%): 1,090.
Lump Sum \$11,967.

JOB NO.	79-11	
DATE	5/23	[79
BY	<u> </u>	Cocary .
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SHEET NO 10/19

SUBJECT FLOOD PROFILE.

CLIENT C.O.E.

(Wage Rates from "Means" - Include off P).

II, FLOODPROOFING - 35'x50' House (Perim: 170 feet).

A. DRAINS & SUMP PUMP

- 1. Break up slab around cellar floor

 Rent air compressor: 2days @\$50/day: \$100.

 1 Worker for 2days: 16 hrs @ \$14.60/kr: 234.

 \$334.
- thend excevate trench, remove matilifrom cellar, load of haul away

 Assume: 2 man crew @ \$31.50/hr

 Trench z'dpx z'uzl @ bese,

 1:2 sides

1/2(4+2) 2 = 6 sq. ft. Inside perim, 2 from wall; 154 Volume = 924/27 = 34 c.y.

Assume: 2 laborers can dig 12 cy/day
2 " move 24 cy/day

3 days diaging @ \$ 31.50/hrx 6 hr/day: 756

Dump Truck (664) @ \$135 / day = 135 Payloader: Hrs@\$34/day : 136 \$1,405.

31 cy. @ \$6.00/cy (met'l) = \$204.

12 hrs. labor @ \$14.60/hr = 175.

8-18

JOB NO	79-162	
DATE	5/25,79	
BY	RK Gagley	-
CH'D I	Y / KETAL	

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

_	BHEET NO. 19/58
<u>ال</u> _ عادر	TSEELO
BUBJECT _	FLOCO PROOFING
	C.C.E

4. 6" V.C.P.
154 L.F. @ \$4/LF (including installation) = \$616.

5. Replace Concrete H'x H/12 +hk x154 1.f./27 = 7.6 c.y. @ \$110/6y = 837

6. Sump Pump, Hose, Install Outlet 275

7. Clean-up, replace tiles, etc.
12 hrs @ \$14.60/hr = 175

TOTAL \$4,021.

B. WATERPROOF WALLS

1. Excevete Trench Around House

Assume: 3.5' wd x 6'dp

Outside penim: 194'(2'from wall)

Volume = 3.5' x6' x194'/27 =

150 c.y. @ \$2.80/cy = \$420.

2. Clean Walls, Apply Waterproofing 6'n+x 170' perim = 1,360 5.f. @ \$1/5.f. = 1,360

3. Backfill & Compact Trench 150 cy. @ \$3.60/cy: 540.

4. Restore Site (Cleen Up, Restore shrubs, ferces, etc.). 340

B-19 TOTAL IA & IB - \$6,681.

SHEET NO 20/38

JOB PITSFIELD

SUBJECT FLOOD PROFINICS

CLIENT C.O.E.

C. BLOCK UP WILLDOWS

- 1. Remove Exist. Windows
 Assume: 8 windows 2hrs. labor/ea.
 16 hrs. 6 \$14.60/hr = \$234.
- 2. Material Concrete Blocks 6 windows x 16"x32"ea/144 = 28 s.f. @ \$0.70/sp = 20.
- 3. Installation
 Assume: 1 Bricklayer # 4 16.55/hr
 1/2 hrs x 8 x 416.55/hr = \$222.00
 Use Some As For Removal 234.

TOTAL PART IA: \$4,021
IB: 2,660.

TOTAL PART I : 47,169

JOB PIT OF ICLD
SUBJECT FLOOR POOLING
CLIENT CO.E

II. RAISING FOUNDATION

A EXISTING CELLAR

- Disconnect & Restore Lines
 Hesting, sewer, water, gas, electric,
 Telephone etc. lines must be cut &
 extended, junction boxes installed, etc.
 L.S. Estimate 41,700.
- 2. Underpinning Jacking

 Mobilization \$700

 Elevate attructure Cut walls,

 jack, place cribbing, etc.

 Labor: H days-2mechanics, alaborers

 Mechanics: \$21/hr x + x6x2

 Laborers: \$14.60/hr x + x6x2

 Equipment Rental For Duration

 1,200

 \$41,645.
- 3. Maxing

 Pour or lay new foundation fix all

 stainways, fix or pour new floor slabs

 Assume:

 -3' foundation x170' perim (new folnowalls) @ \$3/sf

 Stainways of slab (Assume

 1 maxin of helper)

 \$33,25 (combined)/hr x 3 days x8 798

JOB PHIST IN 22 38
SUBJECT FLOOD HOTTING
CLIENT C.O. E

- 4. Materials
 Lines, pipes, brick/block/concrete, etc.
 Estimate L.S. \$225.
- 5 Restoration
 Clean up, shrubs, lawn, plaster cracked walls, restore basement, repair porches, etc. 1,150.
- L. Care for Occupants
 Hausing for duration, moving of
 Valuables, etc.
 Assume:
 30 day duration & 465 play 1,950
 TOTAL \$11,998.

TOTAL PART IA: 11,998.

TOTAL ANYS JAFE, IL GIA: \$19,167.

JOB PITTOFIELD
SUBJECT FLOW PROFING
CLIENT C.O.E

(02]2

CASE "A" - DRAINAGE SYSTEM WITH SUMP PUMP (See pp. 18 & 19, IA).

Raw Cost:

10% Contingencies:

General Contractor Ottel P (10%):

4 4,021. (pg.19)

402.

402.

44,825

54,825

Engineering & Survey (20%) 965 \$5,790.

COST / PERIM. FT = \$5,790/170' = \$34.00 /ft.

CASE 'B"- DRAHAGE SYSTEM, SUMP PUMP & WATERPROOFING (See pp. 18 & 19, IA & FB)

Raw Cost:

10% Contingencies:

General Contractor OHEP (10%):

Engineering & Survey (20%)

1,603.

\$ 9,620.

COST / PERIM . FT. = \$9,620 /170 = \$56,60 /ft.

JOB PITT SFIELD
SUBJECT FLOOD PORTING
CLIENT C.O.E.

CASE "C"- DRAINAGE SYSTEM, SUMP PUMP, WATER PROOFING & BLOCK UP WILLOOUS (SEE PP. 18 - 20! IA, IB & IC) \$ 7,169, (25,20) Row Cost 1 10% Contingencies: General Contractor OH&P(10%) 717. \$ 10,323 COST / PERIM: FT. = \$10,323 /170 = \$60,70 /ft. CASE "D" - RAISE HOUSE 3 FEET (SEC AP. 21 - 22) ILA). \$ 11,998. Raw Cost: 10% Contrigencies:

General Contractor OH&P(10%): 1,200

\$ 14,398. Engineering & survey (10%) 1 1,440

Lump SUM - 5,838.

JOB NO	79-16-21
DATE	5/23/79
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HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON MASSACHUSETTS

JOB PITTS FIELD

BUBJECT FLOOR PROFING

CLIENT C.O.E.

(Wage Rate: From "Means"-Include OH. & P.)

I. FLOODPROOFING - 50'x 62' HOME (Perim = 224 feet)

A. DRAINS & SUMP PUMP

1. Breaking up slab around cellar floor

Rent or compressor @ \$50/day x3days \$150,

1 worker for 3 days: 24 hrs. @ \$14.00/hr 350.

(Includes cleanup) \$500.

2 Hand excevate tranch, remove matil from cellar; load & haul away
Assume: 2 man craw 6731.50/hr.
Tranch zidp. x z'wd & base,
1:2 Sides

1/2 (4+2) 2 = 6 sq.ft. Inside perm. 2'from wall:208' Volume =1240/27 = 46 cy.

Assume: 2 laborers candig 12 cylday
2 " move 24 cylday

4 days digging @ \$31.50/crewxehroldey

\$1,008. 504

Dump Truck (6 cy) @ \$ 135/day Pey loader: 4 hro @ \$34/hr

135. 136. \$ 1,783,

3. Backfill - crushed stone 46 cy. @ \$6 /cy. (mat'l) 16 hrs labor @ \$14.60/hr

\$ 276. 234

JOB N	٠	7-)	- :	<u>67</u>	. 1	
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CH.D	BY.		P	57		

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON: MASSACHUSETTS

BHEET NO. 26/38
JOB PITS FIRED
BUBJECT Flood Proofing
CLIENT COE

4. 6" V.C.P. 208 1.f. @ \$4/1.f. (including installation) \$832.

5. Replacing Concrete
4'x 4/12 thk x208 l.f./27 =
10.3 c.y @ \$110/cy
1,133.

6. Sump Pump, Hose, Install Gutlet 400.

7. Clean up, Replace tiles, etc.
10 hrs. @ \$14.10/hr. 234

TOTAL \$5,392

B. WATERPROOF WALLS

1. Excensic Trench Arand House
Assume: 3.5' wd x u'dp.
Outside perim: 250' (z'from well)
Volume: 3.5 x u x 250/27:
194 cy @ 4z.80/cy 4543.

2. Clean Walls, Apply Water proofing 6' ht x 224' penm = 1,792 st. @ \$1./s.f. 1,792.

3. Backfill Trench w/ Compaction
194 cy @ \$3.60/c.y. 698

4. Restore Site (Clean up, Replace Shrubs, Fences, etc.) 340.

C. BLOCK UP WINDOWS

- 1. Remove Exist Windows Assume: 12 windows @ 2hrs. labor/ea. 24 hrs. @ \$14.00/hr. \$350.
- 2. Material Concrete Blocks
 12 windows x 16"x 32"/ea/144 =
 42 s.f. @ 40,70/s.f 29.
- 3. Installation
 Assume: | Bricklayer & 4 18,55/hr.

 1/2 hrs x12 x \$18,55 = \$334.

 Use Same As For Removal 350

 TOTAL \$729

TOTAL PART IA; \$ 5,392.

"IB; \$ 5,392.

3,373.

"IC; 729

TOTAL PART I; \$ 9,494

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

JOB PIT SPIELD

SUBJECT FLOOD FROM

CLIENT C.O.E.

IL, RAISING FOUNDATION

A. EXISTING CELLAR

- Heating, sewer, water, 925, electric, telephone etc. lines must be cut of extended, junction boxes installed, etc.
- 2 Underpinning Jacking

 Mobilization

 Estimate to cut walls, underpin, jack

 up, place cribbing, etc.

 Labor: 5 days 2 mechanics, 3 laborers

 Mechanics: \$21/hr x5 x8x2

 Laborers: \$1,680

 Laborers: \$146/hrx5 x8x3

 Equipment Rental For Duration

 \$5,332.
- 3. Masony
 Pour or lay new foundation, fix all
 stainways, fix or pour new floor slab.
 Assume:
 3' foundation x224 perm (new fdn.
 walls) & #3./sf. #2 pic.
 Stainways & slab (Assume
 1 mason & helper)
 #33.25 (combined)/hr x4daysx8 1,064.
 #3,080.

JOB PITTS FIELD
SUBJECT FOR PROFING
CLIENT C.O.E

- 4. Materials
 Lines, pipes, brick/block/concrete, etc.
 Estimate L.S. 4450.
- 5 Restoration
 Clean up, thrubs, lawn, plaster cracked walls, restore basement, repair porches, etc.

 1,450
- 6. Care for Czeupants
 Howaing for duration, moving of
 Valuables, etc.
 Assume:
 30 day duration @ 465/day 1,950

 TOTAL \$13,962

TOTAL PART II A: 13,962 TOTAL PART II A: 13,962 TOTAL PARTS IA IB, IC & IIA: 4 23,456

JOB PITT SPIELD

<u>CO515</u>

CASE "A" - DRAINAGE SYSTEM WITH SUMP PUMP (See PP. 30 \$ 26 1 IA)

Raw Cost : 10% Contingencies:

\$ 5,392 (pg.26) 539 \$ 5,931

General Contractor's OH&P (10%): 539 \$ 6,470.

Engineering of Survey (20%): 1,294

\$ 7,764.

COST / PERM. FT. = \$7,764 /224 = \$ 34,66 /ft.

CASE "B"-DRAINAGE SYSTEM, SUMP PUMP EVATER PROOFING (See pg, 26, IA \$ IB)

> PAW COST: 10% Contingencies:

\$ 8,765 (PG. 24). 877. 514J. P. C

General Contractor's OH & P (10%): 877. \$ 10,519.

Engineering & Survey (20%): 2,104

4 12,623

COST / PERIM FT. = \$12,628/224 = \$56,86 / Ft.

HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON MASSACHUSETTS

JOB PITSFIELD
SUBJECT Flood Andring
CLIENT C.O.E.

CASE "C"- DRAINAGE SYSTEM, SUMP PUMP,

VIATERPROOFING & BLOCK UP WILLDOWS

(See Ap. 29-27: IA, IB & IC).

Raw Cost ; 10% Contingiencies :

\$ 9,494 (pg.27) \$ 10,443.

General Contractor OH&P (10%):

949 • 11,392,

Engineering & Survey (20%):

4 13,670

COST / PERIM. FT. = \$18,470/224 = \$61.03 /ft.

CASE "D"- RAISE HOUSE 3 FEET. (See pp 28:29, IIA).

> Rew Cost: 10% Contingencies:

ط اعربی (مع عور) اعربی المعربی
General Contractor OH&P(10%)

1,396. \$15,858 1,396 316,754

Engineering of Survey (10%)

1,675

LUMP SUM - +18,429

HH HAYDEN. HARDING & BUCHANAN. INC. CONSULTING ENGINEERS BOSTON. MASSACHUSETTS

JOB PITTE FILLD
SUBJECT Flood Proping
CLIENT C. C. F

34,

COST OF FLOOD BARRIERS

- Include: excavation, sleeve placement, granting, backfill, patching pavement of grassed area.

 Estimate Lump Sum /Ea. 4100.
- 2. Flood Berner Frame

 Day a 10' Section will be 10' Lg. z'Ht.

 Use 1/z" plywood on a frame of z'k3"'s.

 Frame top4 bottom, ends 4 insert a

 Stress diagnal.

 Total Lenoth (2x3's): [2(10')+2(2')+10.2']

 (a) \$ 11 LF
- 3. Plywood Barrier
 10'x2' @ \$0.60/5= 12.
- 4. Waterproofing
- 5. Joint Seel & Bettern Apron
 Joints: 2' 6, \$6,50/LF

 Bottom: 10' @ \$0,80/LF

 \$
 1.63.

\$ 163/10' = \$16,30/LF.

JOB N	≀0	79	-]	62		_
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HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON, MASSACHUSETTS CLIENT (.O.E.

SHEET NO 33/38

DEMOLITION COSTS

BASED UPON \$0.10/CF

Sour	HWEST B	Beak	4		
			ALL RESIDENCES	EXCEPT	AS NOTED.
REALH	- 2			100YR.	SPF
114 B	rker Ro.				1900
Cadu	zul Ro (f	ARMOE	y Garace)		3900
106	CAOWELL	_ Ro.		5000	5000
111	11	11			2300
1160	u	11	•	2400	2400
120	н	41		2100	2100
123	11	\$į.			1400
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207	*1	11		2400	2400
229	и	11			2300
185	\$1	Ħ			1500
233	H	ŋ		2000	2000
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HAYDEN, HARDING & BUCHANAN, INC CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

JOB PITTED
SUBJECT FLOOD STYDY
CLIENT CO.F.

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	REXH	- 2 (Car	(0'7		
	4 6	EELIDALE	Ave.	100 YR	<u>50e</u> 500
	655	Jackson s	.		2700
**	427 433 472 484 490 573 576 586 590	H H H H H H H H H H H H	" (BIGI "H") " (MICONALO'S) " (SHELL STATION " (SULICCO STATION " " " " " " " " " " " " " " " " " " "	7200 1300 13200 1200 1200 1200	3400 1400 1300 1300 1300 1300 1300 1400
	595-5 595-5 595-5	8 11 597 11 11	" (Apart) " (Body Shop) " (The Shop)	1800 5000 7200	1900 1800 4000 5000 7200
	* work	المكسمود	REMOVAL OF VLIDER	records (Gas Tauks.

DATE 5/23/79
BY R. FAGIEN
CHID BY D. GAQUIN

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

JOB PITTELO STUDY
CLIENT C. G. F.

Southwest Beauch		
REACH Z (CONT'O)	·	
ZO WOODLEICH RO. 30 " "	100 Ye.	5. <u>P.F.</u> 1800 2400
9-11 ZOAR ST (APACT.)	2700	2700 2700
WEST BRAILCH		
REACH Z BOI COLUMBUS AVE (RIVERSIDE CAFE)	100 Ye.	<u>5.P.F.</u> 2300.
73 R DEWEY AVE.	1900	1900.
REACH 3 300 COLUMBUS FUE (WEST SIDE FISH MKT))	Z600
97 DEWEY AVE (MILTOLL'S PLACE) 102 " " 107 " " 113 " " 114 " " 117,119,4121 " " 125 " " 129 " " 18-35	500	500 9800 2300 2400 1400 3700 8200 1400 2300

HAYDEN, HARDING & BUCHANAN, INC CONSULTING ENGINEERS BOSTON, MASSACHUSETTS

SHEET NO 36 58

JOB PITTSFIELD

SUBJECT FLOOD STUDY

CLIENT CO.E.

	WEST BRANCH	
	REACH 3 (CONT'O)	Ye. 5, P.F.
	135-137 DEWEY AVE. (APART.)	4000.
	1 - 1 H	3700
	143-145 " (Apart)	3700
	1 -1 7	3700
	155	2600
	159 " (COMM.) 49X	Z600 2600 2600 2600 2600 2600 2600 2600
	165 " (COMM.) 49x	2300 2300
	175 " " "	2300 2300
	56 JOHN ST. (BEEKSTIEE BAKERY) ZBIO	0 28100
	62-68 R " " (COMM.)	9000
	90 " "	4000
	104 "	3500
		·
*	198 LINDER ST. (AMPCO STATION) 20,00	0. 20,000.
	50 Prospect St.	4100.
	311 50 -1-011 00-	Liana
	24 Southern Ave. (Apart.)	4000
	1 = C (LD(Let 1.)	∞1€

^{*} INCLUDES REMOVAL OF LINDEGERAND GASTANKS.

HH HAYDEN. HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON. MASSACHUSETTS

JOB PITTELD

SUBJECT FLOOD STUDY

CLIENT C.O. F

WEST BRANCH		
REACH H		
72-74 DANFORTH AVE. (APART).	100 YE.	<u>5.P.F.</u> 4000.
87 " " 84 " " 88 "	z3∞,	, 00ES 00ES 008
90-92 " (APART) 94-96 " (APART)		2600 4500
135 " (HOURT)		6900 3100
3 DANIELS AVE. EXT		1600.
275 Dewey Ave.		1600. 2 3 00
287 " " 291 " "		4500 3300
II LEIDHOLD PL.		23 <i>0</i> 0 8 6 00
LINDEN ST. (BOB'S AUTOMOTIVE) 202-204 " " (APART.)	1600	6900
52 Pack ST. (BODY SHOP)		9000 3100
77 " " (COMM)	8400	8400
161 SEYMORE ST. (RICHMOND BAKERY)		22600

B-37

JOB NO. 70 -1102	_
DATE _5/22/79	<u>. </u>
BY R.K. BACTLEY	_
CH'D BY D. GAQUIN	_

HH HAYDEN, HARDING & BUCHANAN, INC. CONSULTING ENGINEERS BOSTON. MASSACHUSETTS

JOB _ 	5HEET NO 3-38
SUBJECT _	FLOCO STUDY
CLIENT	COE

WEST	- Ber	1-1C+			
REAL	<u>+ 4</u>	(CONT	·'c)	100 YR	<u> 5.P.E.</u>
201	ueller	Que	<u>.</u>		3400
21-23		μ	(Apart.)		7100
25-27		u			7100
3)	, Ll	44		5100	5100
64					3100
72	44	ş i			1600
76	JŁ	H			1600
, 5Z	ч	¥			1600
860	H	11	•		1600
89	н	Ħ			4000
90	ы	U			1900
91	*{	11			2600
92	н	£1			1500
93	<u>r</u> t	F!			2600
94	ч	4			4500
95	4	st			4000.
					,
89 1	watco	uat s	st. (Jeery's	(AFE)	3400
115-12	3 "		" (com)	,	10800
127-13	3 "		" (COMM.)		9600
160 R	N		ji.		1600

